

GeoMat Testing Laboratories,

Soil Engineering, Environmental Engineering, Materials Testing, Geology

April 1, 2020

Project No. 20105-01

- TO: Sri Jayaram Foundation, Inc. 6549 Pimlico Place Eastvale, California 92880
- SUBJECT: Soil Infiltration Report Update, APN 1016-331-05-0000, 4.83 Acres, 12594 Roswell Avenue, City of Chino, County of San Bernardino, California
- REFERENCE: City and County Engineering and Testing, Inc. "Basic Infiltration Testing Report, Proposed Sri Sai Mandir Center, Approximately 4.83 Acres, 12594 Roswell Avenue, City of Chino, County of San Bernardino, California." Project No. J&P2018044.DRI.RPT, Report Dated August 22, 2018.

As requested, we have updated the above referenced soil infiltration report. The purpose of the update is to transfer, to the new provided plan, the previously reported information by City and County Engineering. City and County Engineering is no longer in business.

For easy reference, the previously prepared report by City and County Engineering is attached. There are no changes in the findings, conclusion and recommendation of the previous report except for the following:

- 1. New site plan which supersede the previous site plan. The new plan depicts the exploratory boreholes and soil infiltration tests previously conducted by City and County Engineering.
- 2. New Project Description and usage provided by project representative.

New Project Description

The proposed development is located on a 4.83 acre site at 12594 Roswell Avenue, Chino, CA in the unincorporated area of San Bernardino County. The site is bordered by Roswell Avenue at the East and Walnut Ave at the North. The proposed project is to construct about 32,400 square foot multipurpose building to serve as both a place of worship as well as a facility for various community events & activities. The proposed development also includes about 4,500 square feet of caretaker quarter.

Usage of Proposed Building:

The first level is designed to serve as the main 270- seat congregation area for the purpose of worship and prayer. There will also be a kitchen facility for cooking and a dining hall located adjacent to the main congregation hall at the first floor, as well as. classrooms for the youth,

At Mit

Art Martinez

Staff Engineer

multipurpose meeting rooms, administrative offices and prayer/meditation rooms. A detailed site plan is attached with this document.

The second level will house a prayer hall where devotees can view the idols and perform rituals. There will also be three classrooms for youth to learn about music, dance, yoga, education, etc.

The facility will also be designed to offer spaces for community events and activities. Both the larger hall or the smaller multipurpose rooms and classrooms will function individually for community services such as health fairs, counseling sessions, job search assistance, environmental awareness campaigns, community pantry, food drive, etc.

If you should have any questions regarding this report, please do not hesitate to call our office. We appreciate this opportunity to be of service.

Submitted for GeoMat Testing Laboratories, Inc.

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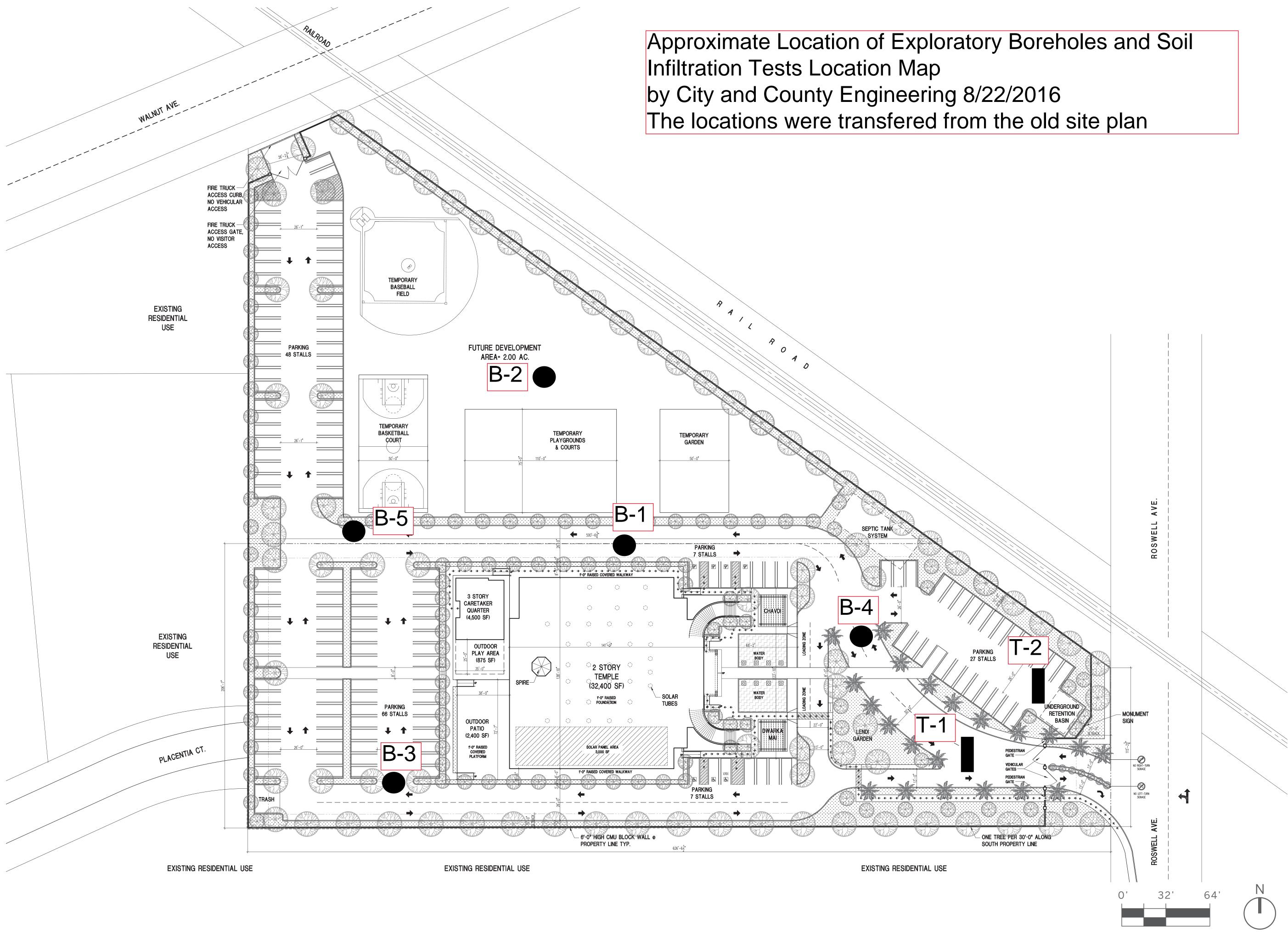
Haytham Nabilsi, GE 2375 Project Engineer, Exp. 12/31/2020



Distribution: (1) Addressee

Attachments:

Plate 1Site PlanAppendix ASoil Infiltration Report by City and County Engineering and Testing





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Contact: Keyur Maru 714.390.0525 www.sajnidesign.com

Client:

Sri Jayaram Foundation, Inc.

6549 Pimlico Place Eastvale, CA 92880

Contact: Arunasri Reddy 951.544.5832

Project:

Sri Sairam Mandir

12594 Roswell Ave. Chino, CA 91710-3036

Revisions:

110 1131	0115.	
01	Conceptual Drawings	11.20.2019
02	Conceptual Drawings v2	11.26.2019
03	Conceptual Drawings v3	12.03.2019
04	Conceptual Drawings v4	12.05.2019
05	Conceptual Drawings v5	12.15.2019
06	Conceptual Drawings v6	12.24.2019
07	Conceptual Drawings v7	01.20.2020
08	Conceptual Drawings v8	03.16.2020

Stamp:



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Drawing Data:

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Issue Date	03.16.2020
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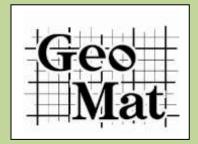
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Sheet:

Site Plan



Appendix A



CITY & COUNTY ENGINEERING AND TESTING, INC.

2324 S. Vineyard Ave., Suite B, Ontario, CA 91761-7764, (909)-930-5868

BASIC INFILTRATION TESTING REPORT, PROPOSED SRI SAI MANDIR CENTER, APPROXIMATELY 4.83 ACRES, 12594 ROSWELL AVENUE, CITY OF CHINO, COUNTY OF SAN BERNARDINO, CALIFORNIA,

APN: 1016-331-05-0000

August 22, 2018 Job #J&P2018044 DRI.RPT

Prepared For: SRI SAI RAM MANDIR 12594 Roswell Avenue CHINO, CA 91710

Prepared By: CITY & COUNTY ENGINEERING AND TESTING INC. 2324 S. Vineyard Ave. Suite B Ontario, CA 91761-7764 (909)-930-5868

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City and CountySoil Engineering

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CITY & COUNTY ENGINEERING

AND TESTING, INC.

2324 s. Vineyard Ave., Suite B, Ontario, CA 91761-7764

August 22, 2018 Job #J&P2018044 DRI.RPT

SRI SAI RAM MANDIR 12594 Roswell Avenue CHINO, CA 91710

Attention: Mrs. Arunasri Reddy. Project Manager Subject: BASIC INFILTRATION TESTING REPORT, IN GENERAL ACCORDANCE WITH ASTM 3385-03 TEST METHOD PROPOSEDSRI SAI RAM MANDIR, APPROXIMATELY 4.83 ACRES, 12594 ROSWELL AVENUE, CITY OF CHINO, COUNTY OF SAN BERNARDINO, CALIFORNIA,

APN: 1016-331-050-0000

Reference: Your Work Authorization and Contract dated August 03, 2018

Introduction

This report provides a summary of the geotechnical engineering services conducted to support evaluation of the feasibility of infiltration at the subject site. The purpose of our services was to complete two (2) in-situ infiltration tests utilizing the double-ring infiltrometer to evaluate the feasibility of infiltration for disposal of stormwater runoff following the falling head method.

Project Description

We understand that an infiltration trench/swale will be utilized to capture storm runoff for on-site disposal for the proposed Sri Sai Ram Mandir Center

Scope of Services

City and County Engineering and Testing was retained to provide geotechnical engineering services to support the project. Our scope of work consisted of the following specific tasks:

- 1) Complete two (2) infiltration tests at the site utilizing the double ring infiltro-meter. The tests were completed in general accordance with the falling head method.
- 2) Complete data analysis.

- 3) Preparation of this report summarizing our findings, conclusions, and recommendations. The report includes:
 - Site plan showing the location of infiltration tests and exploratory trench.
 - Summary of log of conditions observed at the testing locations.
 - Discussion of the results of insitu infiltration testing.
- A discussion of the surficial soil and anticipated groundwater conditions at the site.
- Evaluation of the feasibility of infiltration.
- Recommendations for in-situ infiltration rate.

Existing Site Conditions

The site is located in the southwesterly San Bernardino County. The property consists of the irregular-shaped parcel of 4.83 acre is located at 12594 Roswell Avenue within the City of Chino, California. Based on our site reconnaissance, the site is vacant, now. Southeast of the site was previously developed with a single family home and detached garage, which was later used for Armstrong nursery. Most of the site was used for nursery use. The, structure along with garage, plants and goods were since have been removed from the site leaving a stockpile of crushed rock in the southeast corner. The site is bounded to its north and northeast, south, east and west by chain link fence, partly block wall and wood fence. Rail road tract bordered to its north and northeast. There are few residential and industrial structures located around the subject site. No drainage course is located within the site or close by. The site has general slope towards south and southeast.

Groundwater

Groundwater study is not within the scope of this work. However, no groundwater was encountered in the exploratorytrenches to 15 feet depth.

Based on the California Department of Water Resources and local water company's website; the depth of groundwater at the site is more than 50 feet.

Please note that the potential for rain or irrigation water locally seeping through from adjacent elevated areas and showing up near grades cannot be precluded. Our experience indicates that surface or near-surface groundwater conditions can develop in areas where groundwater conditions did not exist prior to site development, especially in areas where a substantial increase in surface water infiltration results from landscape irrigation. Fluctuations in perched and static water elevations are likely to occur in the future due to variations in precipitation, temperature, consumptive uses, and other factors including urbanization and development. However; it is not likely to be less than 100 feet.

Subsurface Soils

The subsoil encountered in our exploratory borings and Infiltration trench during the exploration consists of **young alluvium**, brown to olive gray, fine silty sand (SM), silt lenses, poorly graded with grass-vegetation, house hold trash and debris to about 5-feet below the existing ground, slightly moist to moist, and very loose. The underlying soils below 5-feet to about 10-feet were found to be olive gray, fine silty sand (SM) and sandy silt (ML), slightly moist to moist and loose to firm. The sub soils between 10-feet to the end of our borings to a maximum depth of 40-feet were olive gray, fine sandy silt (ML) and fine silty sand (SM), poorly graded, moist and medium dense. Generally, the sub soils are very loose in the upper 5-feet and medium dense below 5-8 feet.

Based on the laboratory test results, the subsurface soils in foundation zone consist of mostly fine poorly graded silty fine sand (SM), possess relatively low cohesive properties, are highly susceptible to hydro consolidation and low in expansion potential.

Groundwater or hard bedrock strata were not encountered in any of our exploratory borings/trenches to a maximum depth of 40.0 feet below the existing ground. Information, based on the local water district, the depth of groundwater in the vicinity of the site should be 50-feet or more.

Test Method and Findings

Two infiltration tests were conducted at 5.0 and 8 feet below ground surface, in native soil. Based on the results of this study, infiltration of stormwater at the site is feasible. The following summarizes the result of the infiltration feasibility study and the recommended field infiltration rate for use in design.

Trench excavation for infiltration testing was conducted utilizing a track mounted DEEREJBE- 310 extended hoe backhoe on August 11, 2018. The bottom of the test trenches werecut level to the desired infiltration depth of 5.0 and 8 feet below the ground surface. The soil profile is described in the form of Exploratory Trench Logs, see Appendix B.

Infiltrometer Device

The double-ring infiltrometer test method consists of driving two open cylinders, one inside the other, into the ground and then partially filling the rings with water to a fixed point. The water is added at the constant mark at every time interval. The volume of water added each time interval is equal to the measure of the volume of liquid that infiltrates into the soil. The volume of water infiltrated during the time intervals can be converted into an infiltration velocity (in³/hr). The incremental infiltration velocity within the inner test cylinder is equivalent to the infiltration rate (in/hr).

Infiltration Test Result

City and CountySoil Engineering

Based on the (minimum) test result, water infiltration rate stabilized at 1.5 *inch per hour or 3.81 cm/hr.* for the tests that were conducted 5.0 and 8 feet below ground surface, see Appendix C. This result is raw test result.

Factors of Safety

Based on *Worksheet "H" in the Technical Guidance Document* for Water Quality Management Plans prepared for The County of San Bernardino Area wide Storm water Program dated June 07, 2016, the minimum safety factor for this suitability assessment is 1. The design engineer should complete Worksheet "H" to determine the Total Safety Factor for the BMP. Minimum safety factor should not be less than 2, but may be higher at the discretion of the design engineer and acceptance of the plan reviewer.

Conclusions/Recommendations

- In our opinion, water infiltration at the site is feasible. Filter fabric should be used whenever aggregates are placed against native soils.
- Infiltration water should not be allowed to saturate pavement and concrete structures sub grade soils.
- The planned infiltration system should extend vertically into native soil. The designer should review the attached geotechnical log for soil classification.
- Please note that soils in infiltration areas should not be subject to compaction during construction.
- The proposed system by the civil engineer should be constructed and maintained in accordance with manufacturer guidelines.

An important consideration for infiltration facilities is that, during construction, great care must be taken not to reduce the infiltrative capacity of the soil in the facility through compaction by heavy equipment or by using the infiltration area as a sediment trap.

Infiltration facilities should be constructed late in the site development after soils (that might erode and clog the units) have been stabilized, or should be protected (by flagging) until site work is completed.

Infiltration facilities should be sited with the following guidelines:

INFILTRATION	FACILITY SETBACKS
Setback From	Distance
Property Lines and Public Right of Way	5 feet
Foundations	15 feet or within a 1:1 plane drawn up from

	the bottom of foundation
Slopes	H/2, 5 feet minimum (H: is slope height)
Private drinking water wells	100 feet

Ferrous metal pipes should be protected from potential corrosion by bituminous coating, etc. We recommend that all utility pipes be nonmetallic and/or corrosion resistant. Recommendations should be verified by soluble sulfate and corrosion testing of soil samples obtained from specific locations during construction.

If applicable, four to six inch diameter with locking caps observation well(s) extending vertically into the system's bottom is suggested as an observation point. Observation well(s) should be checked regularly and after large storm events. Once performance stabilizes, frequency of monitoring may be reduced.

City & County Soil Engineering should observe the basin excavation. Additional laboratory testing including but not limited to grain size analysis, sand equivalent, sulfate content, etc should be conducted during construction.

Use of this Report

This report was prepared for the exclusive use of the owner and their consultants for specific applications to the proposed site. The use by others, or for the purposes other than intended, is at the user's sole risk.

The findings, conclusions, and recommendations presented herein are based on our understanding of the project and on subsurface conditions observed during our site work. Within the limitations of scope, schedule, and budget, the conclusions and recommendations presented in this report were prepared in accordance with generally accepted geotechnical engineering principals and practices in the area at the time the report was prepared. We make no other warranty either expressed or implied.

We appreciate this opportunity to provide geotechnical services on this project and look forward to assisting the Project Team as the design progresses. Please call our office if you have any questions or comments regarding the information contained in this report, or if we may be of further services

Submitted for **City and County Engineering and Testing Inc**.



Zen Bhatia, RCE #36150, License Expired on 6/30/2020

Distribution: [3] Addressee

Attachments:	Plate 1	Site Photos
	Plate 2	Index Map
	Plate 3	Topographic Map
	Plate-4	Aerial Map
	Plate 5	InfiltrationTest Location Map
	Appendix A	Percolation Data/Graphs

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SITE PHOTOGRAPHS



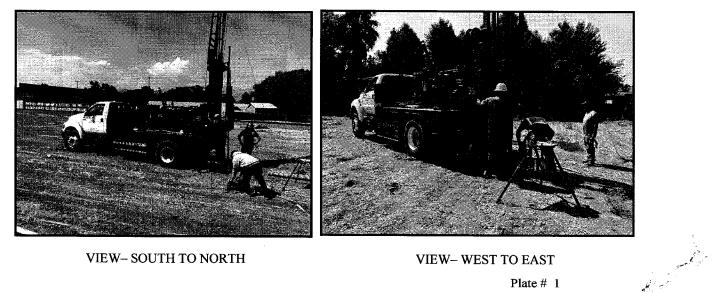
VIEW-EAST TO WEST

VIEW-DOUBLE RING INFILT. TEST



VIEW- NW TO SE



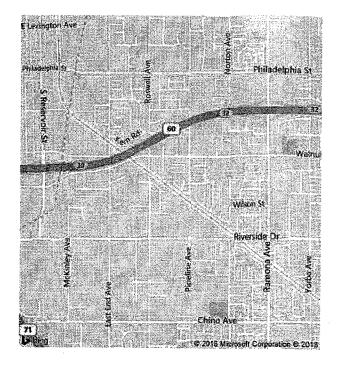


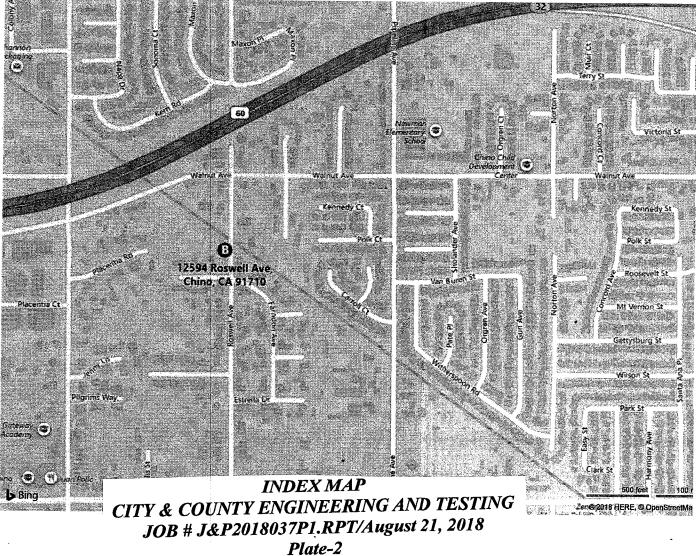
VIEW- SOUTH TO NORTH

VIEW-WEST TO EAST Plate # 1

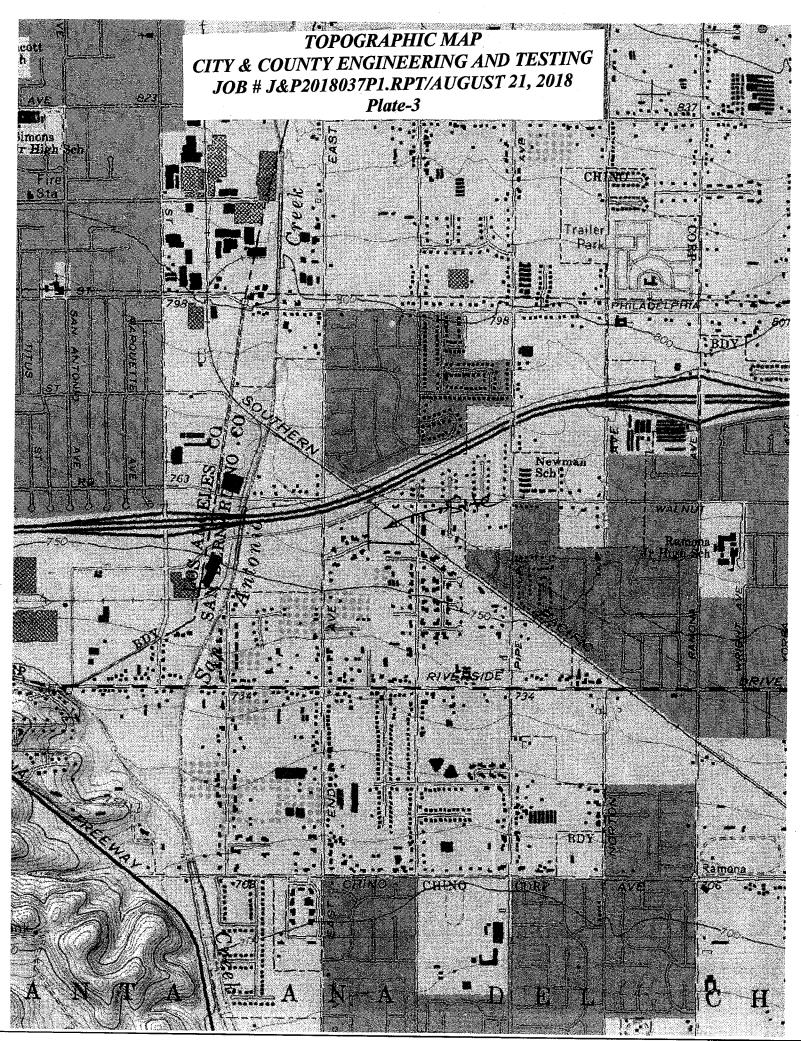
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Notes	
SRI SAI RAM TEMPLE	
JOB #J&P2018037P1	
August 7, 2018	
INDEX MAP	
Plate 2	
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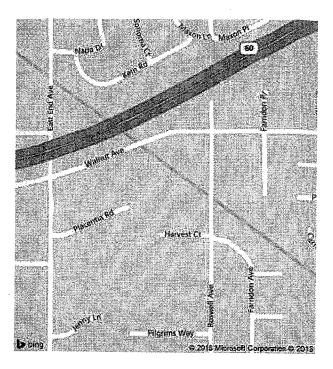
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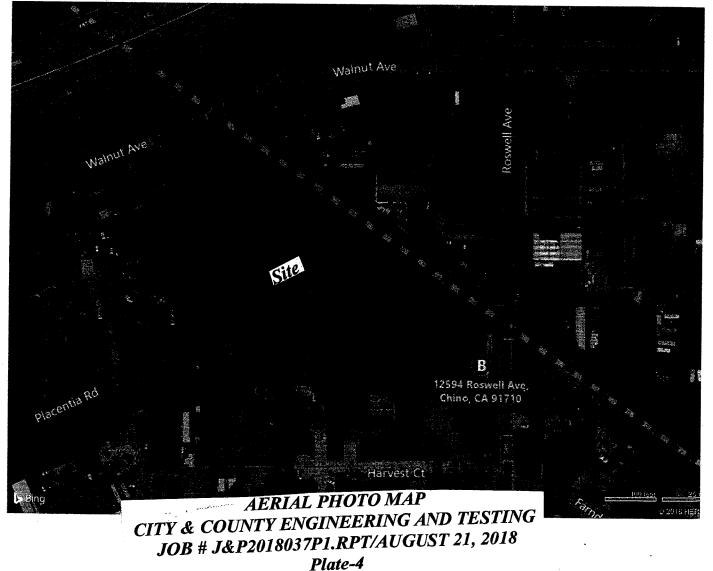


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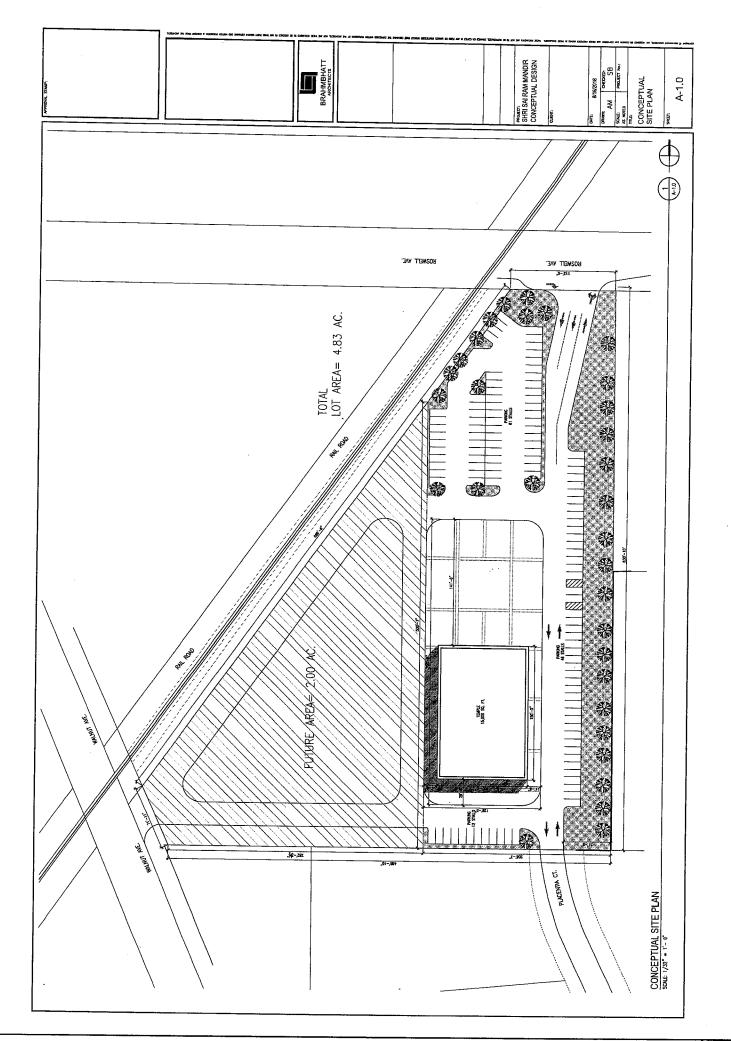
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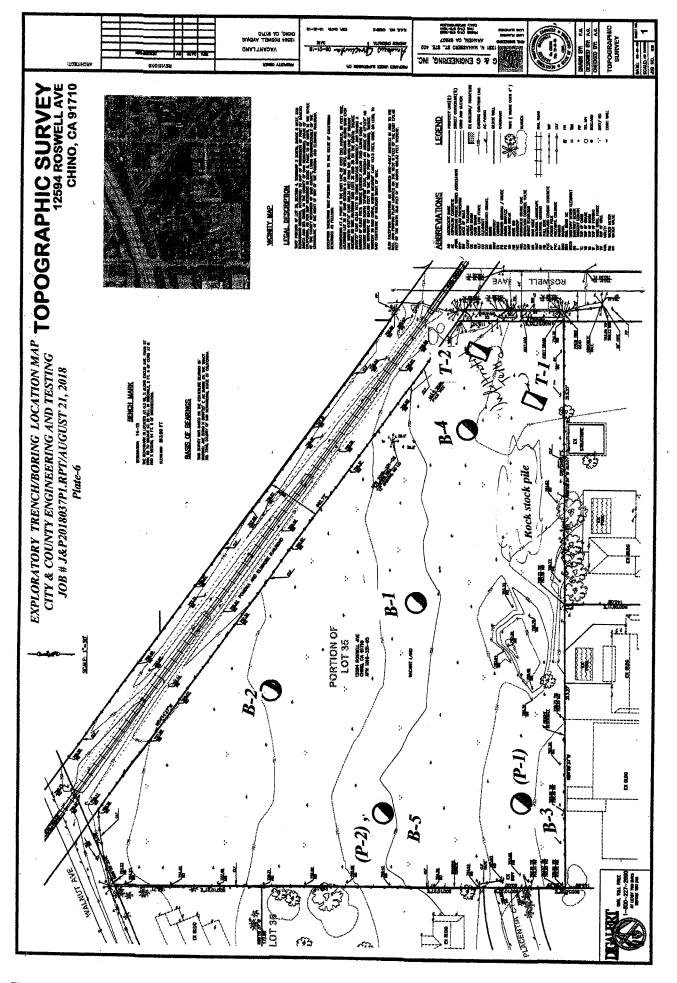
SRI SAI RAM TEMPLE JOB #J&P2018037P1 August 7, 2018 AERIAL MAP Plate 3





https://www.hing.com/mone





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REFERENCES

The County of San Bernardino Area Wide Storm Water Program, Technical Guidance Document for Water Quality Management Plans, June 7, 2013.

San Bernardino County Stormwater Program, Model Water Quality Management Plan Guidance, Jun. 9, 2005.

San Bernardino County Regional Geologic Hazard Maps.

San Bernardino County Stormwater Program, Model Water Quality Management Plan Guidance, Jun. 9, 2005.

Riverside County, Stormwater Quality Best Management Practice, Design Handbook, July 21, 2006.

Riverside County, Water Quality Management Plan For Urban Runoff, Santa Ana River Region, Santa Margarita River Region, September 17, 2004

California Stormwater Quality Association, Stormwater Best Management Practice, Handbook, Jan. 2003.

California Department of Transportation, Stormwater Quality Handbook, Project Training and Design Guide, Sacramento, 2000.

California Department of Transportation, Stormwater Quality Handbook, Project Planning and Design Guide, Sacramento, 2005.

Design Handbook for Low Impact Development, Best Management Practices, Riverside County Flood Control, September 2011.

Water Quality Control Plan, Santa Ana River Basin (8), California Regional Water Quality Control Board, Santa Ana Region, 1995,

Carsel, R. F. and R. S. Parrish. 1988. "Developing joint probability distributions of soil water retention characteristics." Water Resour. Res.24: 755-769.

Federal Highway Administration, Urban Design Drainage Manual, Washington DC, 1996

Massmann, JW, Butchart, and S Stolar, Infiltration Characteristics, Performance, and Design of Stormwater Facilities, Final Research Report, Research Project TI 803, Task 12, Washington DOT 2003.

Soilvision Systems, A Knowledge-Based Soils Database, Murray Fredlund, Canada, 2004.

California Stormwater Quality Association (QASCA), California Stormwater BMP Handbook, Infiltration Trench, TC-10 Design Considerations

BMP Handbook, Part B, Planning Activities, Stormwater Mitigation Measures, Watershed Protection Division, City of Los Angeles.

<u>APPENDIX A</u> <u>PERCOLATION TEST DATA/GRAPH</u>

CITY & COUNTY SOIL ENGINEERING AND TESTING

SRI SAI RAN TEMPLE		Constants-			Ring Data	 L	Liquid Container-			
Location: 12594 ROSWELL AVE, CHINO, County of San Bernardino,		Job #Jo DRI	&P 201	8044	Area, A _r (in ²⁾	Depth of Liquid (in)	VolV _r . (in ^{3/} in) #			
Californ	ia			1	Inner Ri	ng: 12" I)	113	18"	1-78.54
Test By:	: HM/	GL US	CS class	SP/GP	Annular	Space: 2	24"D	339	18"	2-176.7
Water Ta	ible Dep	 t	Penetra	ation of R	ings into	Soil (in.)	:	Inner:		Outer:89 F
Date Tes	st: Tape	water use	xd:	80F	pH: K	Ground '	Temp (F): 83 f		at Depth: 12" /78 F
Liquid Le	vel Mainta	ined by us	sing:	(Flow v	alve () F	loat Va	lve () N	íarriotte T	ube (X) Ot	her: Manually
Additiona	l Comment	S::	Soil L						/Weather	
Date Te	sted: 08/1	1/18		fine silt						
Time	Time	Dt	Inner F	ling	Annula	r RING	Liquid	Infiltratio	n Rats. 1:*:*	Remarks
			Elev.:	AH	Elev:,	.AH	Temp	Inner	Outer	1
			H(in)	(in)/Qf	H(in)	(in)/Q	"F"	in/hr	in/hr	
I - Start	8:40	15.00	2.0	2.5	2.0	2,7	78 f	3.47	2.81	
End	8:55		4.5	196.35	4.7	477.0	78 f	1		
2 - Start	9.05	30.00	2.0	2.1	2.0	2.0	78 f	2.91	2.08	
End	9.35		4.1	164.93	4.0	353.4	78 f	1		
3 - Start	9:37	30.00	2.0	1.75	2.0	2.01	78 f	2.43	2.09	
End	10:07		3.75.	137.44	4.0	355.2	78 f	1		
4- Start	10:09	30.00	2.0	1.5	2.0	1.75	78 <i>f</i>	2.08	1.82	
End	10: 39		3.5	117.81	3.75.	309.2	78 f			
5 - Stare	10:40	30.00	2.0	1.25	2.0	1.75	78 f	1.73	1.82	
End	11:10		3.25	98.17	3.75.		78 f			
6 -Start	11:11	30.00	2.0	1.1	2.0	1.50	78 f	1.53	1.56	
End	11:41		3.1	86.40	3.5		78 <i>f</i>			
7 -Start	11:44	30.00	2.0	1.1	2.0	1.5	78 f	1.53	1.56	
End	12:14		3.14.	86.40	3.5	265.0				
8 -Start	12: 18	30.00	2.0	1.1	2.0	1.5	78 <i>f</i>	1.53	1.56	
End	12.48	<u>_</u>	3.1	86.40	3.5	265.0	78 f			
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"Flow. Qf = AH x Vr-**'Infiltration I =(Qf/Ar)/At

Table 1 - Test Data Form for Double Ring Infiltrometer Test; RiversideCounty - Law Impact Development BMP Design Handbook

					Constants-			Ring Data	l	Liquid Container— VolV _r . (in ^{3/} in) #
Location: 12594 ROSWELL AVE, CHINO, County of San Bernardino,				Job #Jo DRI	&P 201		Area, A _r (in ²⁾	Depth of Liquid (in)		
Californ	ia				Inner Ri	ng: 12" I		113	18"	1-78.54
Test By:	HM/	GL US	CS class	SP/GP	Annular	Space: 2	24"D	339	18"	2-176.7
Water Ta	ble Dep	t	Penetra	tion of R	ings into	Soil (in.)	:	Inner:		Outer:89 F
Date Tes	t: Tape	water use	xd:	80F	pH: K	Ground '	Temp (F): 83 f		at Depth: 12" /78 F
Liquid Le	vel Mainta	ined by us	sing:	(Flow v	alve () F	loat Va	lve () M	farriotte T	ube (X) Ot	her: Manually
Additional	Comment	S::	Soil D						/Weather	
Date Tes	ted: 08/1	1/18		fine silty						
Time	Time	Dt	Inner R	<u> </u>	Annula	r RING	Liquid	Infiltratio	n Rats. 1:*:*	Remarks
			Elev.	AH	Elev:,	.AH	Temp	Inner	Outer	
			H(in)		H(in)	(in)/Q	"F"	in/hr	in/hr	
I - Start	8:42	15.00	2.0	2.75	2.0	2,75	78 f	3.82	2.87	
End	8:57		4.75	215.9	4.75	485.9	78 f	7		
2 - Start	9.05	30.00	2.0	2.25	2.0	2.25	78 f	3.12	2.34	
End	9.35		4.25.	176.7	4.25	397.6	78 f			
3 - Start	9:40	30.00	2.0	2.00	2.0	2.1	78 f	2.78	2.19	
End	10:10		4.00.	157.1	4.1	371.1	78 f			
4- Start	10:12	30.00	2.0	1.75	2.0	2.00.	78 f	2.43	2.08	· · · · · · · · · · · · · · · · · · ·
End	10: 42		3.75	137.4	4.00.	353.4	78 f			
5 - Stare	10:45	30.00	2.0	1.5	2.0	1.75	78 f	2.08	1.81	
End	11:15		3.5	117.8	3.75.	309.2	78 <i>f</i>			
6 -Start	11:18	30.00	2.0	1.25	2.0	1.50	78 f	1.74	1.56	
End	11:48		3.25	98.2	3.5	265.1	78 <i>f</i>			
7 -Start	11:50	30.00	2.0	1.1	2.0	1.50	78 <i>f</i>	1.52	1.56	
End	12: 20		3.14.	86.4	3.25	265.1	78 <i>f</i>	_		
8 -Start	12: 25	30.00	2.0	1.1	2.0	1.50	78 <i>f</i>	1.52	1.56	
End	12.55		3.1	86.4	3.25	265.1	78 f			-
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"Flow. Qf = AH x Vr-**'Infiltration I =(Qf/Ar)/At

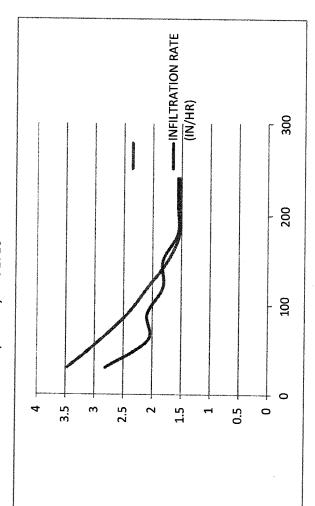
Table 1 - Test Data Form for Double Ring Infiltrometer Test; *RiversideCounty - Law Impact Development BMP Design Handbook*

CITY COUNTY SOIL ENGINEERING AND TESTING Job #J12018044P1-DRI INFILTRATION CURVE (P-1)

ELAPSED TIME (MIN) INFILTRATION RATE (IN/HR)

2.08	2.09	1.82	1.82	1.56	1.56	1.56
2.91	2.43	2.08	1.73	1.53	1.53	1.53
60	06	120	150	180	210	240
	2.91	2.91 2.43	2.91 2.43 2.08	2.91 2.43 2.08 1.73	2.91 2.43 2.08 1.73 1.53	60 2.91 2.08 90 2.43 2.09 120 2.08 1.82 150 1.73 1.82 180 1.53 1.56 210 1.53 1.56 210 1.53 1.56

Site: 12594 ROSWELL AVE., CHINO, CA 91710

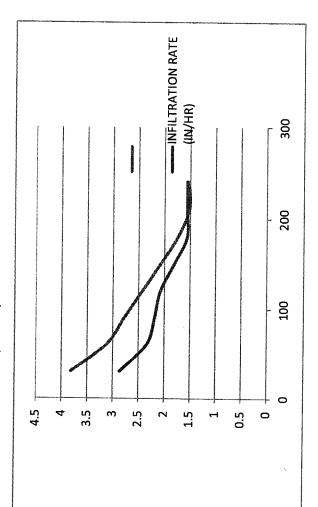


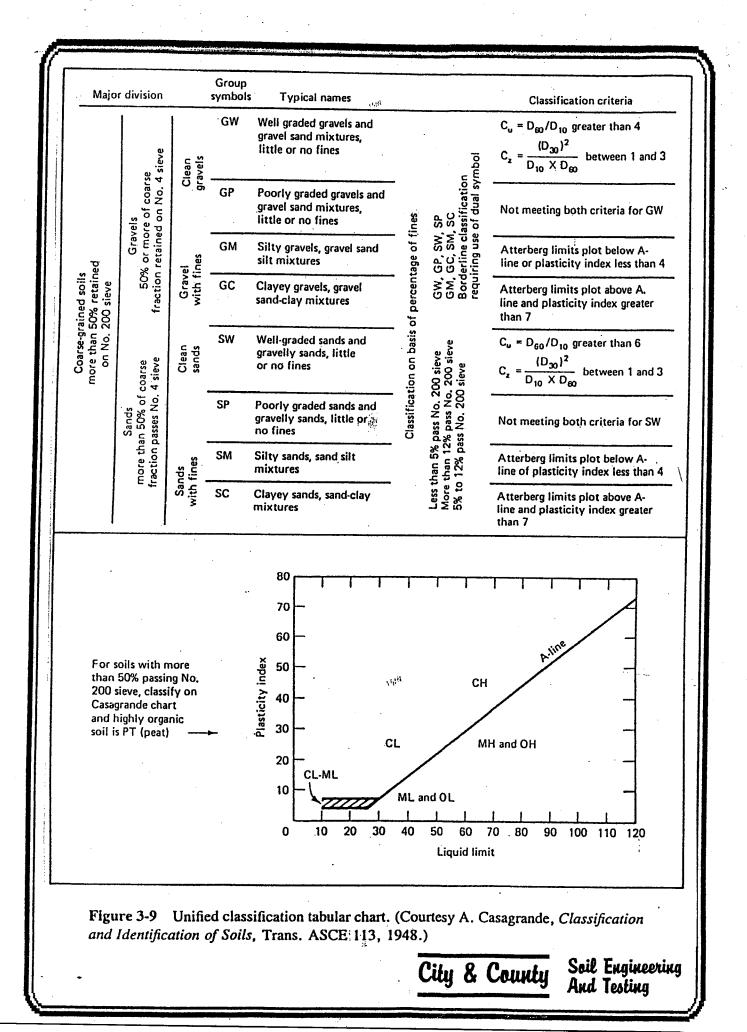
CITY COUNTY SOIL ENGINEERING AND TESTING Job #J12018044P1-DRI

INFILTRATION CURVE (P-2)

ELAPSED TIME (MIN)INFILTRATION RATE (IN/HR)303.822.87603.122.34903.122.34902.782.191202.432.081502.432.081801.741.562101.521.562401.521.56

Site: 12594 ROSWELL AVE., CHINO, CA 91710





Job # J&	P2018037F	91	08/12/18	Client:	RAM MANDIR	
Hole Diameter: 8"		Elev, G.L.	Locatio	n:12594	ROSWELL AVE., CHINO, CA	
Sampling	Method	Drive V	Vt. 140#	CME-4:	5	Logged By: ZB
Drop: 30"	,			Drilling	Co: Gl	EOMAT DRILLING, RIVERSIDE, CA
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type		Earth Materials Description Top Soil: Dense grass-vegetation-12"
					SM	Dark gray, fine silty sand, grass, vegetation, roots s. moi
						very loose
99.1		38	12.0		SM	Brown, fine silty sand, poorly graded, v. moist, med. loos
					<u> </u>	dense, 27% passing #200 sieve
	5					
116.4		37	1.7		SM	Olive gray, fine silty sand, poorly graded, s. moist, med.
		-				dense,
	10					
		8			ML	Olive gray, fine sandy clayey silt, moist, stiff
	15					
N						
		22	-		SM	
					5111	Olive gray, fine silty sand, poorly graded, moist, medium
	20					dense
		19			ML	Olive gray, fine sandy clayey silt, moist, very tiff
	25			-		
						
		15	8.0		SM	Olive gray, fine silty sand, poorly graded, moist, medium
	30					dense, 38% passing #200 sieve

Job # J&P2018037P108/12/18Hole Diameter: 8"Elev. G.L.Sampling MethodDrive Wt. 140#		Client: SRI SAI RAM MANDIR					
		Elev. G.L.	Locatio	n:12594 I	ROSWELL AVE., CHINO, CA		
		Vt. 140#	CME-4		Logged By: ZB		
Drop: 30"				Drilling	Co: GE	OMAT DRILLING, RIVERSIDE, CA	
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	A CARLES AND A CARLES	Earth Materials Description Top Soil: Dense grass-vegetation-12"	
						Dark gray, fine silty sand, grass, vegetation, roots s. moist very loose	
		46 ,			SM	Light brown, fine to coarse silty sand, few gravel, s. moist,	
	5			· · · · · · · · · · · · · · · · · · ·		medium dense, 27% passing #200 sieve	
104.0		17	7.7		SM	Light brown, fine silty sand, moist, med. dense	
	10					27% passing #200 sieve	
-							
	15	9		•	ML	Olive gray, fine sandy clayey silt, moist, stiff	
	20	13		•	ML	Olive gray, fine sandy clayey silt, moist, stiff	
		20			ML	Olive gray, fine sandy clayey silt, moist, very stiff	
	25					End of Boring @ 25 feet Depth	
						No Groundwater Encountered	
		<u> </u>				Boring Backfilled	
	Undisturb						

Job # J&P	2018037P	1	08/12/18	Client: SRI SAI RAM MANDIR					
Hole Diameter: 8"		Elev. G.L.	Locatio	n:12594 F	ROSWELL AVE., CHINO, CA				
Sampling	Method	Drive V	Vt. 140#	CME-4	5	Logged By: ZB			
Drop: 30"				Drilling	Co: GE	OMAT DRILLING, RIVERSIDE, CA			
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	Soil Class	Earth Materials Description Top Soil: Dense grass-vegetation-12"			
					SM	Dark gray, fine silty sand, grass, vegetation, roots s. moist			
						very loose			
	5	31			SM	Brown, fine silty sand, poorly graded, v. moist, med.			
						dense, 27% passing #200 sieve			
	10	14			ML	Olive gray, fine sandy clayey silt, moist, stiff			
	15	9			ML	Olive gray, fine sandy clayey silt, moist, stiff			
	20	13		\blacklozenge		Olive gray, fine sandy clayey silt, moist, stiff			
						dense			
	25	14			ML	Olive gray fine sandy alayoy silt maint with			
				▼	1417	Olive gray, fine sandy clayey silt, moist, stiff			
	30	17			ML	Olive gray, fine sandy clayey silt, moist, very stiff			
D.	Undisturbe		Sample			City & County Soil Engineerin			

BORIN	G LO(G NO.	B-3 (Co	ntinuor	us from	ı 30')				
Job # J&P	2018037P	1	08/12/18	Client:	SRI SAI I	RAM MANDIR				
Hole Diam	eter: 8"		Elev. G.L.	Locatio	n:12594 F	ROSWELL AVE., CHINO, CA				
Sampling]	Method	Drive V	Wt. 140#	CME-4	5	Logged By: ZB				
Drop: 30"				Drilling Co: GEOMAT DRILLING, RIVERSIDE, CA						
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	Soil Class	Earth Materials Description Top Soil: Dense grass-vegetation-12"				
	30	17		•	ML	Olive gray, fine sandy clayey silt, moist, very stiff				
	35	13			ML	Olive gray, fine sandy clayey silt, moist, stiff				
· · · · · · · · · · · · · · · · · · ·						dense, 27% passing #200 sieve				
	40	11			ML	Olive gray, fine sandy clayey silt, moist, stiff				
						End of Boring @ 40 feet Depth				
					-	No Groundwater Encountered				
						Boring Backfilled				
		· · · · · · · · · · · · · · · · · · ·								
	Undisturb Bulk Sam Standard	ple	Sample tion Test			City & County Soil Engineering And Testing				

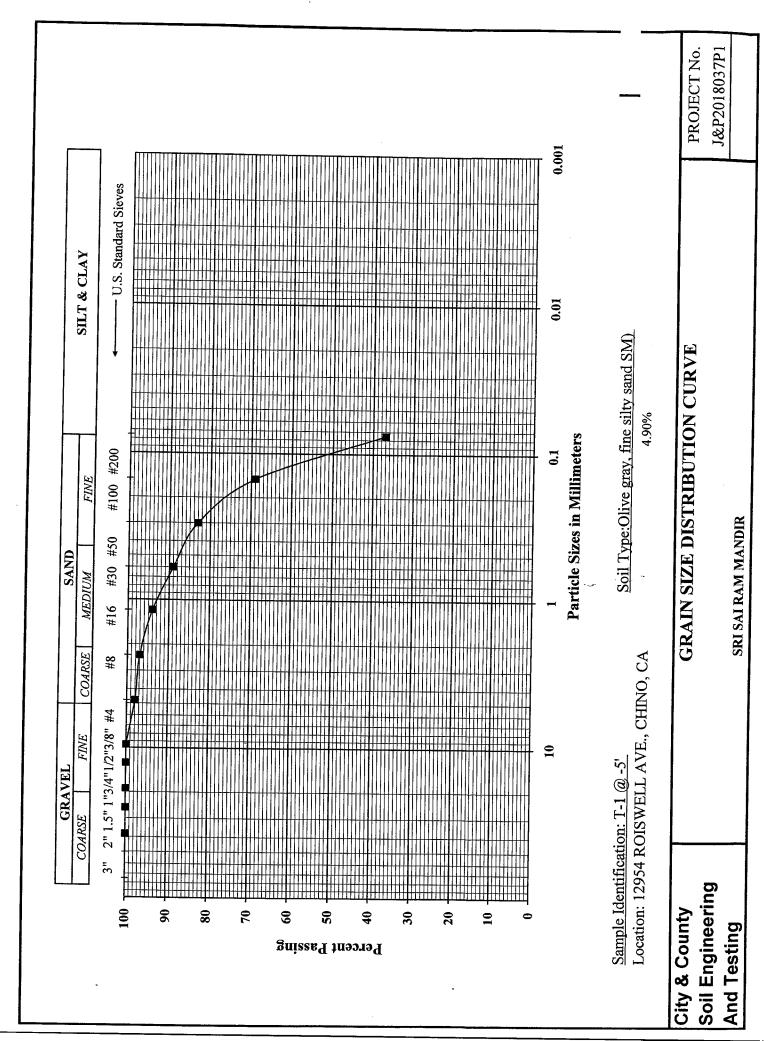
Job # J&F	2018037P	1	08/12/18	Client: SRI SAI RAM MANDIR							
Hole Dian	ieter: 8"		Elev. G.L.			ROSWELL AVE., CHINO, CA					
Sampling	Method	Drive V	Vt. 140#	CME-45 Logged By: ZB							
Drop: 30"						OMAT DRILLING, RIVERSIDE, CA					
Dry	Depth	# of	Moist.		Sample Soil Earth Materials Description						
Density pcf)	(ft)	Blows (ft)	(%)	Туре	Class	Earth Materials Description Top Soil: Dense grass-vegetation-12"					
					SM	Dark gray, fine silty sand, grass, vegetation, roots s. moist					
					-	very loose					
		28			SM	Lt. brown, fine silty sand, poorly graded, s. moist, med.					
	5					dense,					
98.4		33	4.4		SM	Olive gray, fine silty sand, poorly graded, s. moist, med.					
	10					dense, 23% passing #200 sieve					
		·									
			······								
		13	17.8		ML	Olive gray, fine sandy clayey silt, moist, stiff					
	15					57% passing #200 sieve					
			· · · · · ·								
		21			SM	Olive gray, fine silty sand, poorly graded, moist, medium					
	20					dense					
		24	7.3	•	SM	Olive gray, fine silty sand, poorly graded, moist, medium					
	25	İ				Dense, 33% passing #200 sieve					
						End of Boring @ 25 feet Depth					
						No Groundwater Encountered					
						Boring Backfilled					
.	Undisturb	-1.0.				City & County Soil Engineerin					

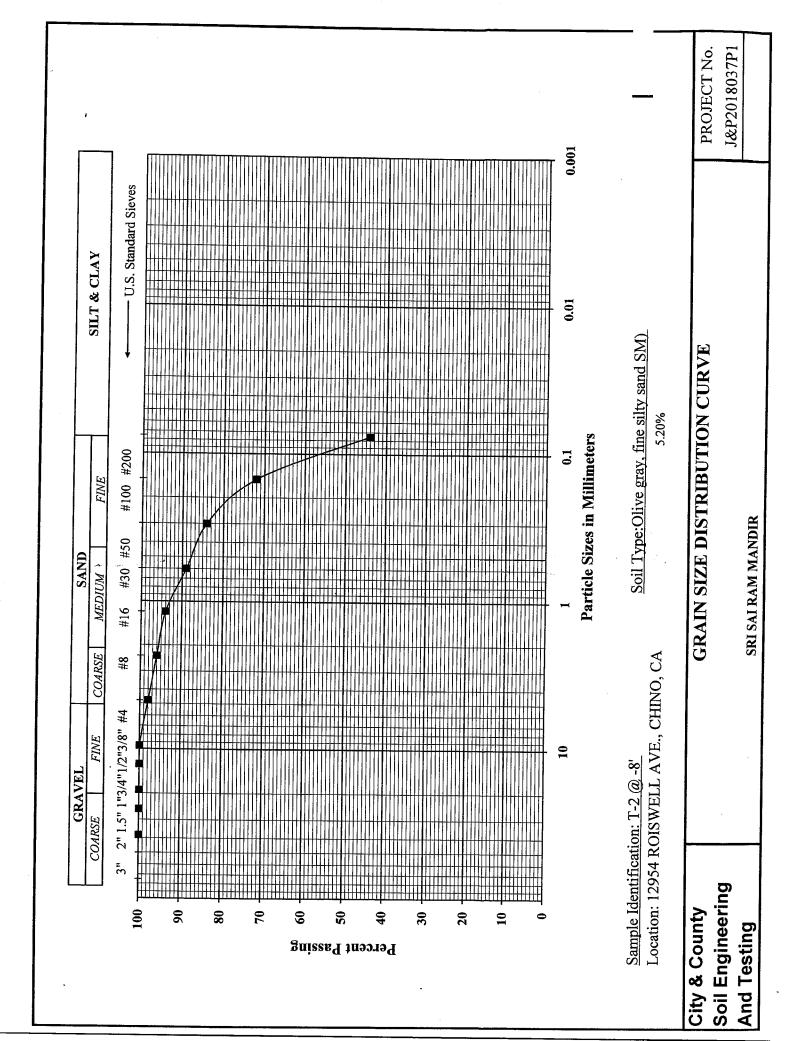
Job # J&P	2018037P	1	08/12/18	Client:	SRI SAI I	I RAM MANDIR					
Hole Diam	ieter: 8"		Elev. G.L.	Location:12594 ROSWELL AVE., CHINO, CA							
Sampling	Method	Drive V	Vt. 140#	CME-4	5	Logged By: ZB					
Drop: 30"				Drilling	OMAT DRILLING, RIVERSIDE, CA						
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	Soil Class SM	Earth Materials Description Top Soil: Dense grass-vegetation-12"					
						Dark gray, fine silty sand, grass, vegetation, roots s. moist					
						very loose					
	5	21			SM	Brown, fine silty sand, poorly graded, v. moist,					
						medium dense, 27% pussing #200 sieve					
91.6	10	14	16.8		ML	Olive gray, fine sandy clayey silt, moist, stiff					
101.0	15	21	16.0		SM	Olive gray, fine silty sand, poorly graded, moist, medium					
						dense					
	20	9		•	ML	Olive gray, fine sandy clayey silt, moist, stiff					
	25	21		•	SM	Olive gray, fine silty sand, poorly graded, moist, medium					
						dense					
	30	29			SM	Olive gray, fine silty sand, poorly graded, moist, dense					

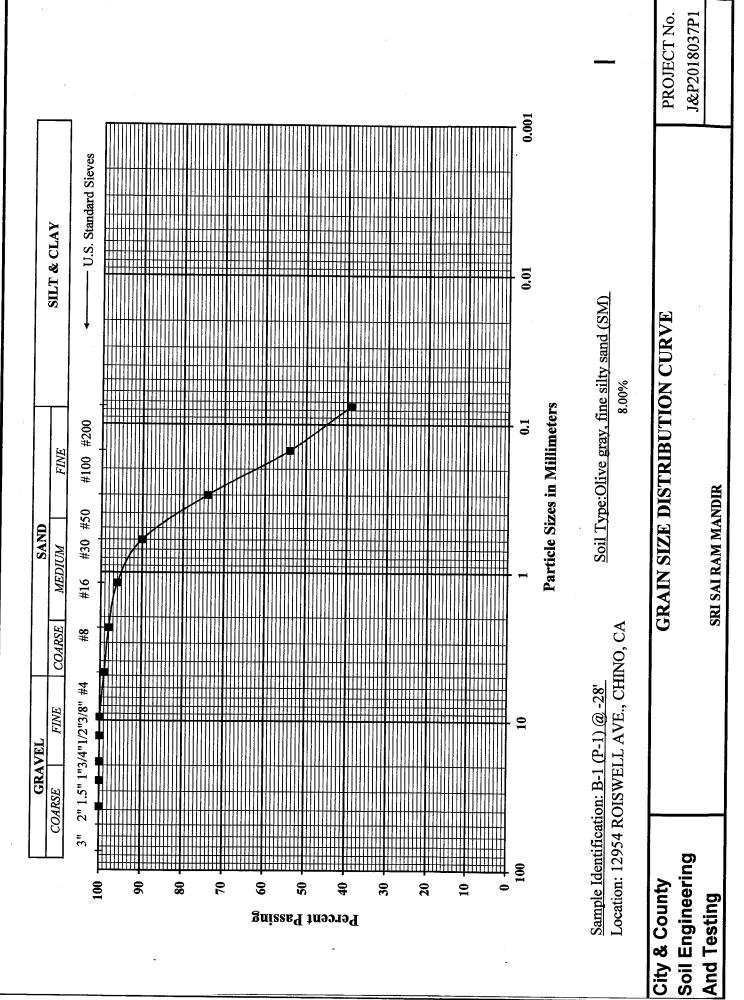
Job # J&P	· · · · · · · · · · · · · · · · · · ·		B-1 (con	1	226556666666666	RAM MANDIR						
Hole Diam												
Sampling 1	<u></u>	Drive W		Location:12594 ROSWELL AVE., CHINO, CA								
Drop: 30"		DIIVE	16 140#	CME-4	<u> </u>	Logged By: ZB						
	n_a					EOMAT DRILLING, RIVERSIDE, CA						
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	Soil Class	Earth Materials Description Top Soil: Dense grass-vegetation-12"						
····	30				SM	Dark gray, fine silty sand, grass, vegetation, roots s. moist						
						very loose						
		16	15.7		ML	Olive gray, fine sandy clayey silt, moist, very stiff						
						53% passing #200 sieve						
	35					· · · · · · · · · · · · · · · · · · ·						
		12	20.0		ML	Olive gray, fine sandy clayey silt, moist, stiff						
						58% passing #200 sieve						
	40											
-						End of Boring @ 40 feet Depth						
						No Groundwater Encountered						
	· ,					Boring Backfilled after Percolation Testing						
			-									
,	Lindictur	ped Ring S	ample									
	Bulk Sam		-		-	<u>City & County</u> Soil Engineerin And Testing						

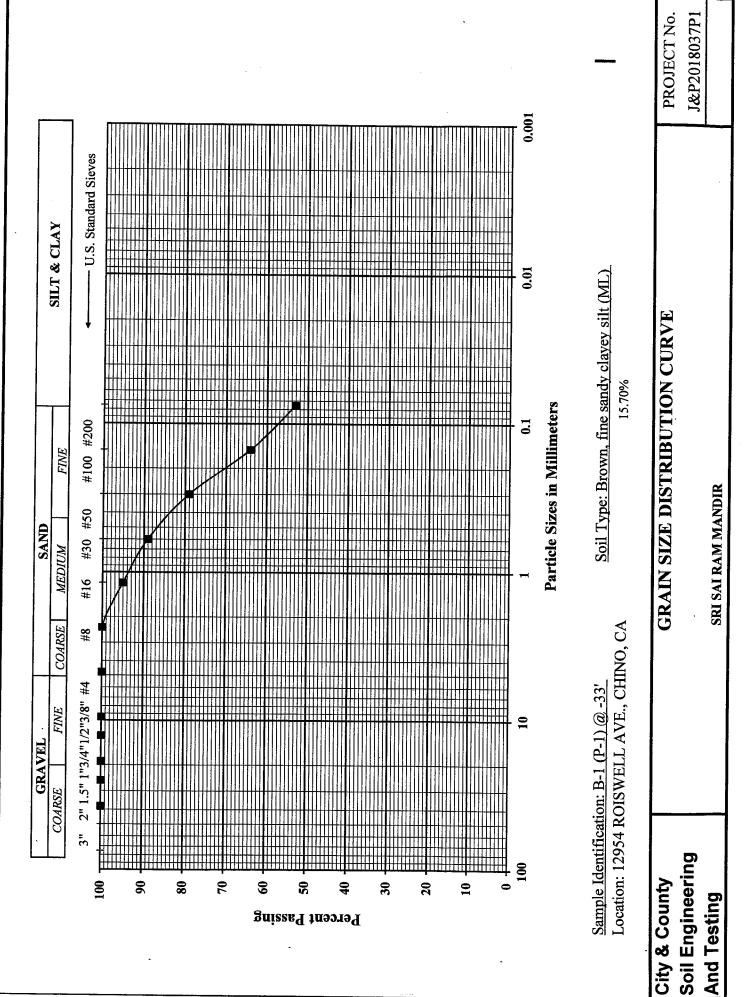
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	N	der	B-5 (con	<u></u>									
Job # J&P		1	08/12/18	Client:	SRI SAI I	RAM MANDIR							
Hole Diam	ieter: 8"		Elev. G.L.	Locatio	Location:12594 ROSWELL AVE., CHINO, CA								
Sampling	Method	Drive V	Vt. 140#	CME-45		Logged By: ZB							
Drop: 30"				Drilling	Co: GE	DATA DRILLING, RIVERSIDE, CA							
Dry Density (pcf)	Depth (ft)	# of Blows (ft)	Moist. (%)	Sample Type	Soil Class	Earth Materials Description Top Soil: Dense grass-vegetation-12"							
	30	29			SM	Olive gray, fine silty sand, poorly graded, moist, dense							
	35	9			ML	Olive gray, fine sandy clayey silt, moist, stiff							
						53% passing #200 sieve							
	40	21			ML	Olive gray, fine sandy clayey silt, moist, stiff							
	,					58% passing #200 sieve							
						End of Boring @ 40 feet Depth							
		-				No Groundwater Encountered							
						Boring Backfilled after Percolation Testing							
			-										
	Undisturb Bulk Sam	ed Ring S ple Penetrat	-			City & County Soil Engineering And Testing							

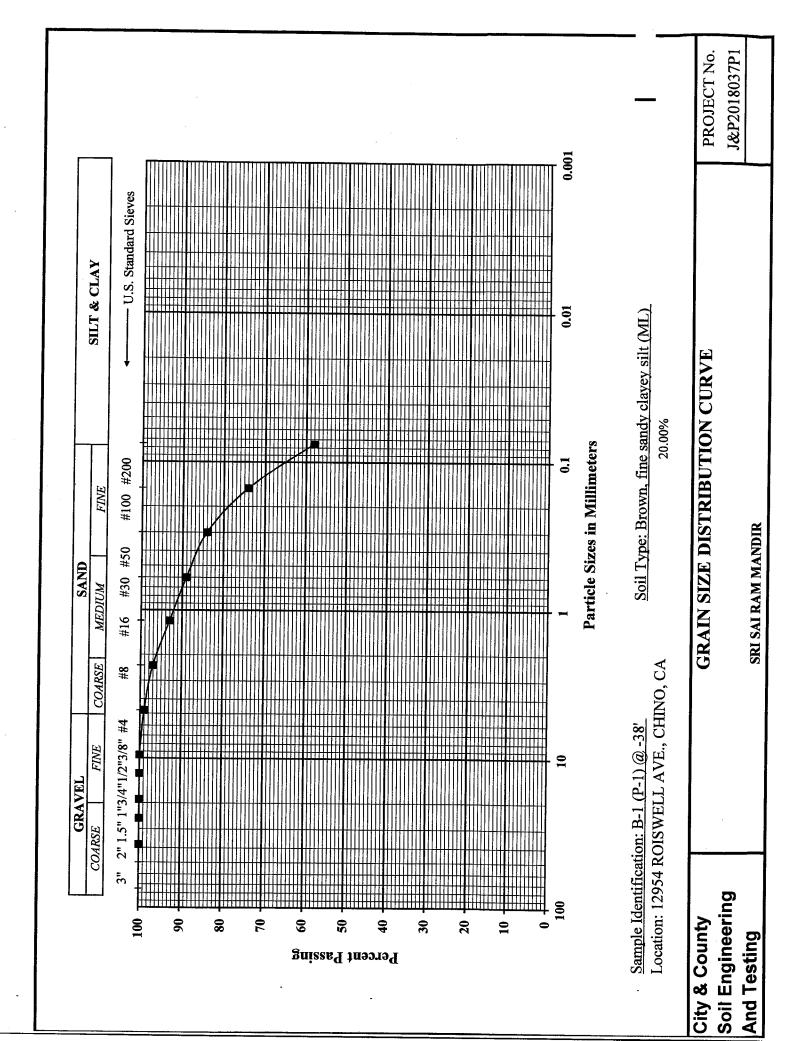


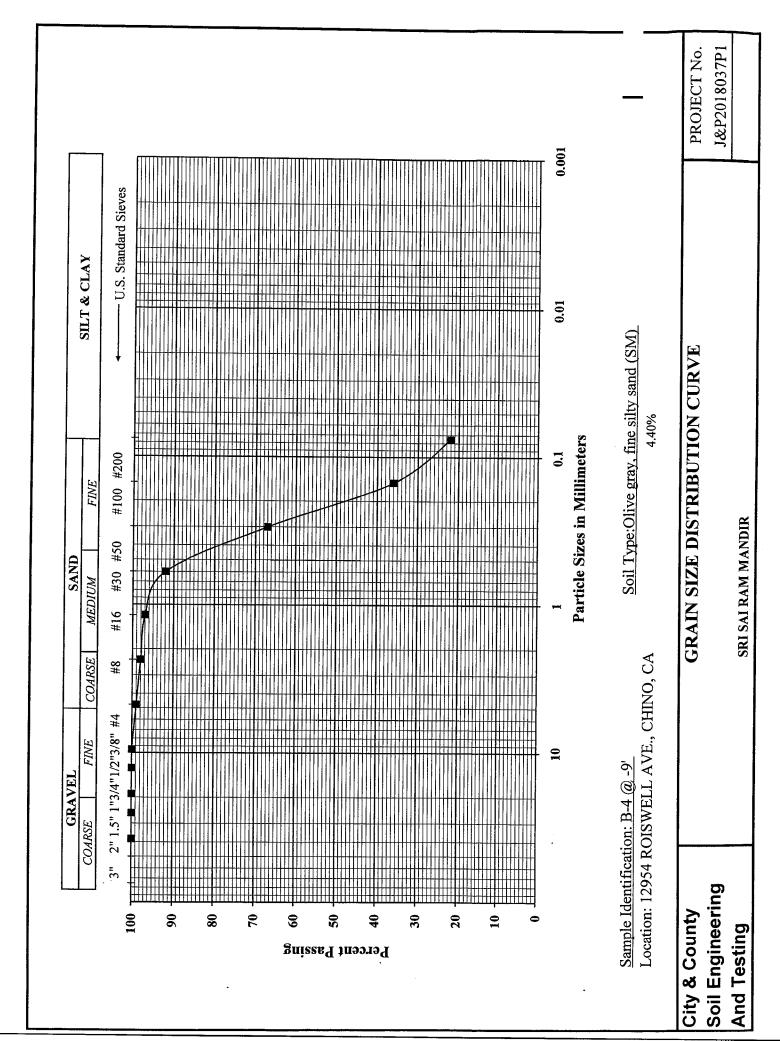


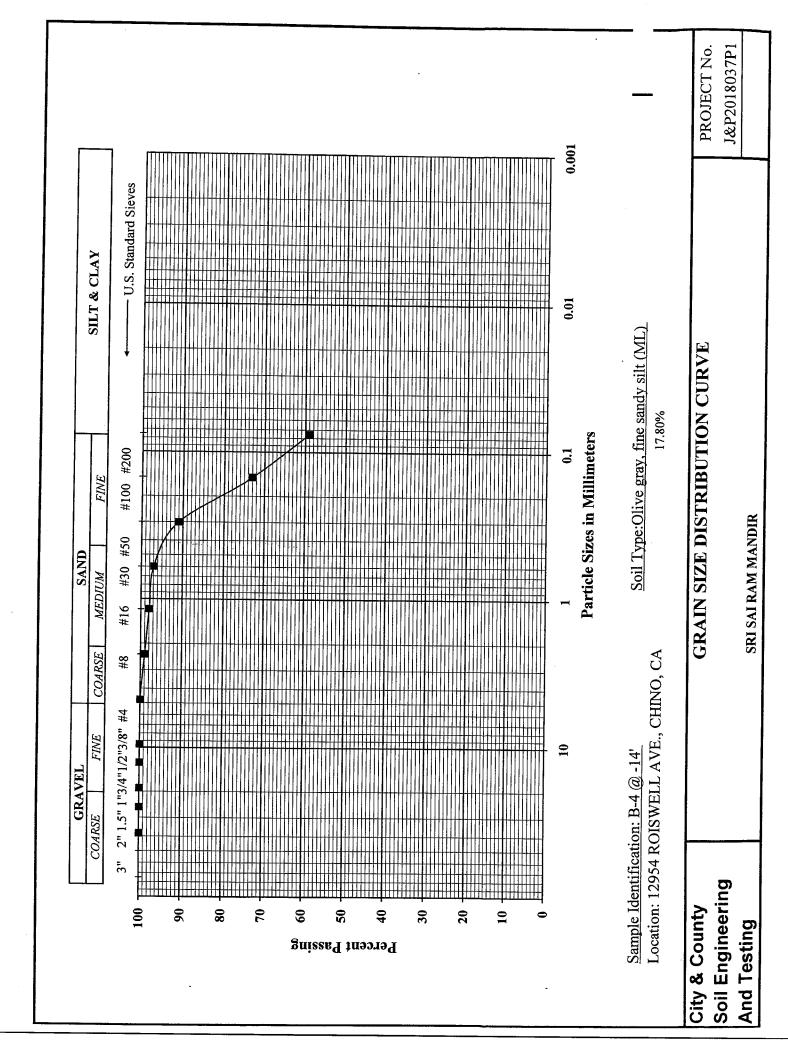


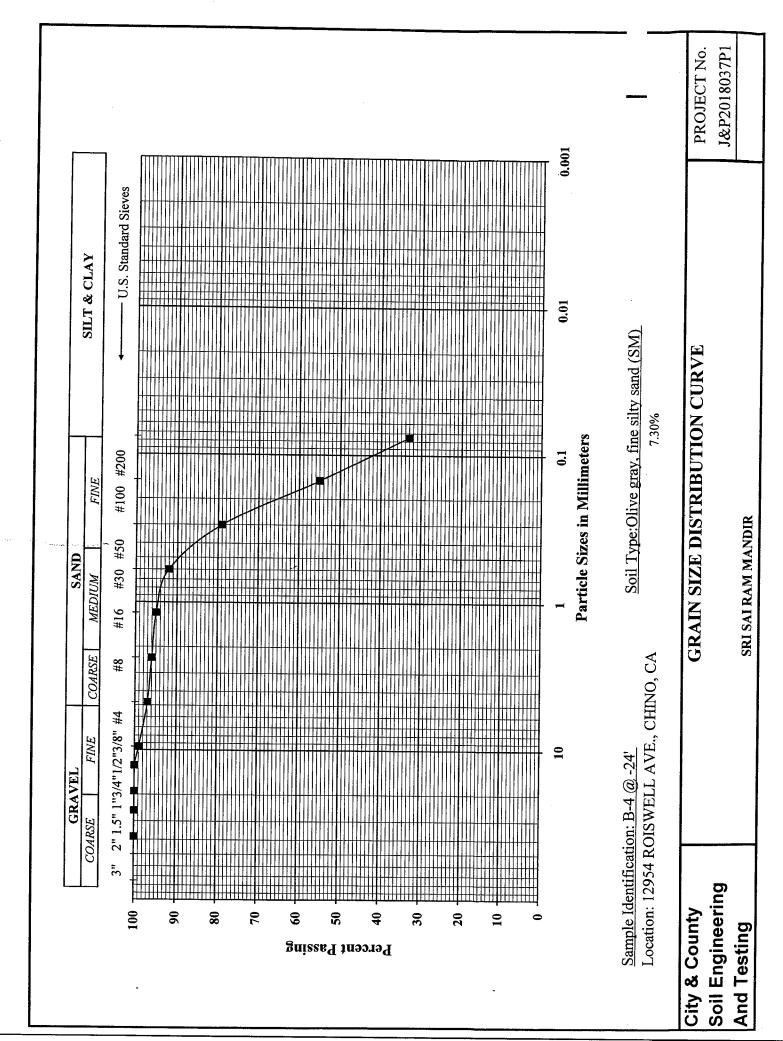


Soil Engineering









Project Name:	L							· · · · · ·	ıble Chlo		·			
Project Name: Project No.:									llected:					
Sample ID:								cted E						
Soil Classification:								ole Te						
son classification:								ed by:						
Specimen No.	<u> </u>				1									
Soil Box Constant (<u>(m)</u>	1 2 1.00 1.00	3	4	1 12	2000						· · · · · · · · · · · · · · · · · · ·	<u> </u>	
Nater Added (ml)	500	15.0 5.0		'								1		
Moisture (%)		11.5 15.4								Q				
Meter Dial Reading	,									1				
Multilier Setting (ol		11.0 4.5 8.3 1K 1K 1K			10	0000				<u> </u>				
Resistance (ohm)		11000 4500 8300												
Minimum Resistivity (ohm-cm)		4500			Ê				1	\				
Temperature (°C)		18.2			Ę,	3000 -						Q		
$\min 15.5 \approx (R_{\min} - r^*/2)$	4 5+T)]/40	48	13 75		ΙĘ					\				
Vater increment: 1	.00-150 ml fe	or large box a	nd 5-1	5 ml	Soil Resistivity (ohm				1	١				
for small box			nu J-1.		5	. 000								
esistivity = Resista	nce X Soil Bo	ox Constant			ĮΞ°	- 000					++-	-/		\neg
arge Soil Box Cons	tant = 6.67 c	cm			ist						N			
mall Soil Box Const	tant = 1.00 c	m			ş						b			
min 15.5 Corrected	d Minimum i	Resistivity to		ľ	l ≣ 4	- 000					-			_
<u>standard</u> Ground T	emperature	of 15.5°C		[ß									
oil Corrosivness		Resistivity (ohm-cn	n)	ľ.									
ery Severely Corro	<u>sive</u>	0 - 900			20	000 -								
everely Corrosive		900 - 2,300												
Ioderately Corrosiv	/e	2,300 - 5,00												
Tildly Corrosive		5,000 - 10,0				0								
ery Mildly Corrosiv	e	10,000 - 100	,000			• •								
eference: ASTM ST haracteristics on C	P 1013 Hitle	d "Effects of S	Soil	1		0.	0	5.0	10.0		15.0	20.0		25.0
	UTTOSION (F	ebruary, 198	9)						Moistu	ire Con	tent (%	6)		
	Dilution	Sulfate Rea	ding		lfate	Cont		CLL						_
Mixing				ppr		T	ent %	Chio	ride Reading			ride Content	на –	
Mixing Ratio		(nnm)		37			.0375		(ppm)		om	%		1
Mixing Ratio 3	Factor 1	(ppm) 125			<u> </u>	1 0	.0375		1		5	0.0005	6.75	4
Ratio	Factor	(ppm) 125								1				-
Ratio	Factor	125	/erage						Averag					1
Ratio	Factor 1	125	/erage						Averag					
Ratio	Factor 1	125			s for (Concr	rete Expo	sed to	-		Soluti	ons		1
Ratio	Factor 1 ACI 318-C	125	Requi	rements		Conci	ete Expo	sed to	Sulfate-Cor	taining]
Ratio 3	Factor 1 ACI 318-C Wat	125 Av 05 Table 4.3.1 ter-Soluble	Requi Sulfa	rements) in				-	taining	N	linimum Des		
Ratio	Factor 1 ACI 318-C Wat sure Sulfa	125 Av 05 Table 4.3.1 ter-Soluble ate (SO4) in	Requi Sulfa	rements) in		ement Ty		Sulfate-Cor Maximum	ntaining w/cm	N	finimum Des pressive Stre		
Ratio 3 Sulfate Expos	Factor 1 ACI 318-C Sure Sulfa Soil,	125 Au 05 Table 4.3.1 ter-Soluble ate (SO4) in % by Mass	Requi Sulfa Wa	rements ate (SO4 ater, ppr) in	с	ement Ty	/pe	Sulfate-Cor	ntaining w/cm	N	linimum Des		
Ratio 3	Factor 1 ACI 318-C Sure Sulfa Soil,	125 Av 05 Table 4.3.1 ter-Soluble ate (SO4) in	Requi Sulfa Wa	rements) in	с		/pe	Sulfate-Cor Maximum	ntaining w/cm	N	finimum Des pressive Stre		
Ratio 3 Sulfate Expos	Factor 1 ACI 318-C Sure Sulfa Soil,	125 Au 05 Table 4.3.1 ter-Soluble ate (SO4) in % by Mass	Requi Sulfa Wa	rements ate (SO4 ater, ppr) in	C No	ement Ty Special 1 II	/pe Type	Sulfate-Cor Maximum by Ma	ntaining w/cm	N	finimum Des pressive Stre		
Ratio 3 Sulfate Expos	Factor 1 ACI 318-C Sure Sulfa Soil,	125 05 Table 4.3.1 ter-Soluble ate (SO4) in % by Mass < 0.10	Requi Sulfa Wa	rements ate (SO4 ater, ppr < 150) in n	C No	ement Ty Special 1 II MS), IS(N	/pe Type	Sulfate-Cor Maximum by Ma	ntaining w/cm	N	finimum Des pressive Stre Mpa (psi) 		
Ratio 3 Sulfate Expos	Factor 1 ACI 318-C Sure Sulfa Soil,	125 Au 05 Table 4.3.1 ter-Soluble ate (SO4) in % by Mass	Requi Sulfa Wa	rements ate (SO4 ater, ppr) in n	C No IP(ement Ty Special 1 II MS), IS(N P(MS),	/pe Type /IS),	Sulfate-Cor Maximum by Ma	ntaining w/cm	N	finimum Des pressive Stre		
Ratio 3 Sulfate Expos Negligible Moderate	Factor 1 ACI 318-C Sure Sulfa Soil,	125 05 Table 4.3.1 ter-Soluble ate (SO4) in % by Mass < 0.10	Requi Sulfa Wa	rements ate (SO4 ater, ppr < 150) in n	C No IP(ement Ty Special 1 II MS), IS(N P(MS), (PM)(MS	/pe _ <u>ype</u> //S),),	Sulfate-Cor Maximum by Ma	ntaining w/cm	N	finimum Des pressive Stre Mpa (psi) 		
Ratio 3 Sulfate Expos Negligible Moderate	Factor 1 ACI 318-C Wat Sure Sulfa Soil, 9 0.1	125 At 05 Table 4.3.1 ter-Soluble ate (SO4) in % by Mass < 0.10 0 to 0.20	Requi Sulfa Wa	rements ate (SO4 ater, ppr < 150) to 150) in m 0	C No IP(ement Ty Special 1 II MS), IS(N P(MS),	/pe _ <u>ype</u> //S),),	0 Sulfate-Cor Maximum by Ma	utaining w/cm ss	N	Ainimum Des pressive Stre Mpa (psi) 28 (4000)		
Ratio 3 Sulfate Expos Negligible Moderate (See Water	Factor 1 ACI 318-C Sure Sulfa Soil, 0.1 0.2	125 05 Table 4.3.1 ter-Soluble ate (SO4) in % by Mass < 0.10 10 to 0.20 10 to 2.00	Requi Sulfa Wa 150	rements ate (SO4 ater, ppr < 150) to 150) to 10,0) in m 0	C No IP(ement Ty II MS), IS(N P(MS), (PM)(MS I(SM)(MS V	/pe [ype //S),],])	0 Sulfate-Cor Maximum by Ma 0.5 0.45	w/cm ss	N	Ainimum Des pressive Stre Mpa (psi) 28 (4000) 31 (4500)		
Ratio 3 Sulfate Expos Negligible Moderate (See Water Severe Very Sever	Factor 1 ACI 318-C Wat Sulfa Soil, 0.1 0.2 e 0.2	125 Av 05 Table 4.3.1 ter-Soluble ate (SO4) in % by Mass < 0.10 0 to 0.20 0 to 2.00 > 2.00	Requi Sulfa Wa 150 1,500 >	rements ite (SO4 iter, ppr < 150) to 150) to 10,0 10,000) in m 0	C No IP(ement Ty II MS), IS(N P(MS), (PM)(MS I(SM)(MS V V + pozz	/pe [ype //S),),))	0 Sulfate-Cor Maximum by Ma 0.5 0.45 0.45	w/cm ss	N Comp	Ainimum Des pressive Stre Mpa (psi) 28 (4000) <u>31 (4500)</u> 31 (4500)	ngth fc,	
Ratio 3 Sulfate Expos Negligible Moderate (See Water Severe Very Sever	Factor 1 ACI 318-C Wat Sulfa Soil, 0.1 0.2 e 0.2 e	125 Av 05 Table 4.3.1 ter-Soluble ate (SO4) in % by Mass < 0.10 0 to 0.20 0 to 2.00 > 2.00	Requi Sulfa Wa 150 1,500 > ral con	rements ite (SO4 iter, ppr < 150) to 150) to 10,0 10,000 crete as) in m 0 000	C No IP(ement Ty II MS), IS(N P(MS), (PM)(MS I(SM)(MS V V + pozz	/pe ype //S),),))	0 Sulfate-Cor Maximum by Ma 0.5 0.45 0.45	w/cm ss	N Comp	Ainimum Des pressive Stre Mpa (psi) 28 (4000) 31 (4500) 31 (4500)	ngth fc,	
Ratio 3 Sulfate Expos Negligible Moderate (See Water Severe	Factor 1 ACI 318-C Wat Sulfa Soil, 0.1 0.2 e 0.2 e	125 Av 05 Table 4.3.1 ter-Soluble ate (SO4) in % by Mass < 0.10 0 to 0.20 0 to 2.00 > 2.00	Requi Sulfa Wa 150 1,500 > ral con	rements ite (SO4 iter, ppr < 150) to 150) to 10,0 10,000 crete as) in m 0 000	C No IP(ement Ty II MS), IS(N P(MS), (PM)(MS I(SM)(MS V V + pozz	/pe ype //S),),))	0 Sulfate-Cor Maximum by Ma 0.5 0.45 0.45	w/cm ss	N Comp	Ainimum Des pressive Stre Mpa (psi) 28 (4000) 31 (4500) 31 (4500)	ngth fc,	

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onsite corrosion potential, soluble sulfate content, and soluble chloride content in soil.

Comments: Sec. 4.3 of ACI 318 (2005) Soil environment is detrimental to concrete if it has soluble sulfate > 1000 ppm and/or pH < 5.5. Soil environment is corrosive to reinforcement and steel pipes if chloride ion > 500 ppm or pH < 4.0.

The information in this form is not intended for corrosion engineering design. If corrosion is critical, a corrosion specialist should be contacted to provide further recommendations.

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