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**Proposed Manufacturing /Warehousing Facility
Preliminary Drainage Study
APN 0298-063-07
County of San Bernardino
August 16, 2017**

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San Bernardino County

Land Development Engineering

Prepared for:

800 Opal, LLC
Attn: Charles Walden
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Newport Beach, CA 92660



LAND DEVELOPMENT DIVISION COMPLETED
BY DAO DATE 9-6-17
REPORT APPROVAL/ADDITIONAL REVIEW
YES ☒ NO ☐ YES ☐ NO ☒

Description

The project site is approximately 35 acres in size and is located along the west side of Opal Avenue between Nice Avenue, to the north, and Colton Avenue, to the south, in the Mentone area of San Bernardino County. The west portion of the site is currently developed with two existing buildings and paved access. The east portion of the site is currently undeveloped with an existing canopy and minor paved access. The site currently drains from east to west at an approximate grade of 2%. The majority of the site's offsite flows enter Opal Avenue that borders the project to the west. From Opal, flows are directed north or south to two existing catch basins that direct flows west to an existing storm drain channel. Ultimately, all flows enter the Santa Ana River to the northwest.

The existing frontages along Nice Avenue, Opal Avenue and Colton Avenue are fully improved with curb and gutter and sidewalk. Therefore, there will be no anticipated flows from the north, west or south to the project area. Neighboring properties in the existing housing tract to the east direct flows east to Beryl Avenue, thus, there are no anticipated flows expected from the east to the subject site.

Proposed development of the site includes grading the easterly undeveloped portion of the site to have a uniform grade from east to west. Proposed permeable, dust-proof, gravel surfacing will be placed in this area for use as outdoor storage. This gravel area is self-treating for water quality treatment as described in the Water Quality Management Plan. The two existing buildings onsite will remain. Minor pavement removals within the existing parking lot along the west portion of the site will occur to provide for ADA access and construction of required landscaped areas. In addition, the existing canopy in the proposed outdoor storage area will be removed. A block wall for screening and landscape buffer is proposed on the north and south perimeter of the project site. Flows from the site will continue as they do historically from east to west. There will be no increase in volume or intensity of flows leaving the site from pre-development to post-development conditions.

Purpose

The purpose of this study is to analyze the flows to and through the site both pre-development and post-development and demonstrate that the post-development flows leaving the site will be less than pre-development flows.

Analysis

To achieve the desired goal the following steps will be taken:

1. Determine the 10, 25 and 100 year pre-development flows.
2. Determine the 10, 25 and 100 year post-development flows.
3. Determine if any onsite mitigation will have to occur in order to have post-development flows that leave the site be less than pre-development flows in both intensity and volume.

Results

1. The 10, 25 & 100 year pre-development flows were determined utilizing the Rational Method per San Bernardino County Hydrology Manual. AES 2015 Software was utilized for the calculations and they can be found in the appendix of this report. The variables used were:

Rainfall Values (per NOAA Atlas 14, Volume 6, Version 2 Map, Figure 4.1):

$$Y_{10} = 0.816$$

$$Y_{100} = 1.41$$

Soil Group (per reference Soil Group Map in appendix, Figure 4.2): A

Pervious Cover Designations:

Mobile Homes
Condominiums
0.4 Dwellings/Acre
Commercial
Public Park

Node 3:

10-year peak flows: $Q_{10} = 14.55$ CFS

10-year time of concentration: $T_{c10} = 12.56$ min

$$\begin{aligned}\text{10-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (14.55 \text{ cfs})(12.56 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{16,448 \text{ cubic feet}}\end{aligned}$$

25-year peak flows: $Q_{25} = 19.54$ CFS

25-year time of concentration: $T_{c25} = 12.56$ min

$$\begin{aligned}\text{25-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (19.54 \text{ cfs})(12.56 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{22,089 \text{ cubic feet}}\end{aligned}$$

100-year peak flows: $Q_{100} = 29.88$ CFS

100-year time of concentration: $T_{c100} = 12.56$ min

$$\begin{aligned}\text{100-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (29.88 \text{ cfs})(12.56 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{33,777 \text{ cubic feet}}\end{aligned}$$

Node 6:

10-year peak flows: $Q_{10} = 17.17$ CFS

10-year time of concentration: $T_{c10} = 12.42$ min

$$\begin{aligned}\text{10-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (17.17 \text{ cfs})(12.42 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{19,193 \text{ cubic feet}}\end{aligned}$$

25-year peak flows: $Q_{25} = 23.08$ CFS

25-year time of concentration: $T_{c25} = 12.42$ min

$$\begin{aligned}\text{25-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (23.08 \text{ cfs})(12.42 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{25,799 \text{ cubic feet}}\end{aligned}$$

100-year peak flows: $Q_{100} = 35.36$ CFS

100-year time of concentration: $T_{c100} = 12.42$ min

$$\begin{aligned}\text{100-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (35.36 \text{ cfs})(12.42 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{39,515 \text{ cubic feet}}\end{aligned}$$

Node 9:

10-year peak flows: $Q_{10} = 14.58$ CFS

10-year time of concentration: $T_{c10} = 12.04$ min

$$\begin{aligned}\text{10-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (14.58 \text{ cfs})(12.04 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{15,799 \text{ cubic feet}}\end{aligned}$$

25-year peak flows: $Q_{25} = 19.33$ CFS

25-year time of concentration: $T_{c25} = 12.04$ min

$$\begin{aligned}\text{25-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (19.33 \text{ cfs})(12.04 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{20,946 \text{ cubic feet}}\end{aligned}$$

100-year peak flows: $Q_{100} = 28.99$ CFS

100-year time of concentration: $T_{c100} = 12.04$ min

$$\begin{aligned}\text{100-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (28.99 \text{ cfs})(12.04 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{31,414 \text{ cubic feet}}\end{aligned}$$

Node 11:

10-year peak flows: $Q_{10} = 8.36$ CFS

10-year time of concentration: $T_{c10} = 5.38$ min

$$\begin{aligned}\text{10-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (8.36 \text{ cfs})(5.38 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{4,048 \text{ cubic feet}}\end{aligned}$$

25-year peak flows: $Q_{25} = 10.53$ CFS

25-year time of concentration: $T_{c25} = 5.38$ min

$$\begin{aligned}\text{25-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (10.53 \text{ cfs})(5.38 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{5,099 \text{ cubic feet}}\end{aligned}$$

100-year peak flows: $Q_{100} = 14.54$ CFS

100-year time of concentration: $T_{c100} = 5.38$ min

$$\begin{aligned}\text{100-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (14.54 \text{ cfs})(5.38 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{7,041 \text{ cubic feet}}\end{aligned}$$

Pre-development Total:

10-year peak flows: $Q_{10} = 54.66$ CFS

10-year volume produced = 55,488 cubic feet

25-year peak flows: $Q_{25} = 72.48$ CFS

25-year volume produced = 73,933 cubic feet

100-year peak flows: $Q_{100} = 108.77$ CFS

100-year volume produced = 111,747 cubic feet

2. The 10, 25 & 100 year post-development flows were determined utilizing the Rational Method per San Bernardino County Hydrology Manual at each individual basin. AES 2015 Software was utilized for the calculations and they can be found in the appendix of this report. The variables used were:

Rainfall Values (per NOAA Atlas 14, Volume 6, Version 2 Map, Figure 4.1):

$$Y_{10} = 0.816$$

$$Y_{100} = 1.41$$

Soil Group (per reference Soil Group Map in appendix, Figure 4.2): B

Pervious Cover Designations:

1 Dwelling/Acre

3-4 Dwellings/Acre

Commercial

Node 3:

10-year peak flows: $Q_{10} = 13.80$ CFS

10-year time of concentration: $T_{c10} = 12.15$ min

$$\begin{aligned}\text{10-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (13.80 \text{ cfs})(12.15 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{15,091 \text{ cubic feet}}\end{aligned}$$

25-year peak flows: $Q_{25} = 18.89$ CFS

25-year time of concentration: $T_{c25} = 12.15$ min

$$\begin{aligned}\text{25-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (18.89 \text{ cfs})(12.15 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{20,657 \text{ cubic feet}}\end{aligned}$$

100-year peak flows: $Q_{100} = 29.69$ CFS

100-year time of concentration: $T_{c100} = 12.15$ min

$$\begin{aligned}\text{100-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (29.69 \text{ cfs})(12.15 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{32,467 \text{ cubic feet}}\end{aligned}$$

Node 6:

10-year peak flows: $Q_{10} = 22.42$ CFS

10-year time of concentration: $T_{c10} = 12.69$ min

$$\begin{aligned}\text{10-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (22.42 \text{ cfs})(12.69 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{25,606 \text{ cubic feet}}\end{aligned}$$

25-year peak flows: $Q_{25} = 29.56$ CFS

25-year time of concentration: $T_{c25} = 12.69$ min

$$\begin{aligned}\text{25-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (29.56 \text{ cfs})(12.69 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{33,761 \text{ cubic feet}}\end{aligned}$$

100-year peak flows: $Q_{100} = 43.95$ CFS

100-year time of concentration: $T_{c100} = 12.69$ min

$$\begin{aligned}\text{100-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (43.95 \text{ cfs})(12.69 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{50,196 \text{ cubic feet}}\end{aligned}$$

Node 9:

10-year peak flows: $Q_{10} = 15.99$ CFS

10-year time of concentration: $T_{c10} = 11.07$ min

$$\begin{aligned}\text{10-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (15.99 \text{ cfs})(11.07 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{15,931 \text{ cubic feet}}\end{aligned}$$

25-year peak flows: $Q_{25} = 20.98$ CFS

25-year time of concentration: $T_{c25} = 11.07$ min

$$\begin{aligned}\text{25-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (20.98 \text{ cfs})(11.07 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{20,903 \text{ cubic feet}}\end{aligned}$$

100-year peak flows: $Q_{100} = 30.99$ CFS

100-year time of concentration: $T_{c100} = 11.07$ min

$$\begin{aligned}\text{100-year volume produced} &= (Q)(T_c)(60 \text{ min})\left(\frac{3}{2}\right) \\ &= (30.99 \text{ cfs})(11.07 \text{ min})(60 \text{ min})\left(\frac{3}{2}\right) = \mathbf{30,876 \text{ cubic feet}}\end{aligned}$$

Post-development Total:

10-year peak flows: $Q_{10} = 52.21$ CFS

10-year volume produced = 56,628 cubic feet

25-year peak flows: $Q_{25} = 69.43$ CFS

25-year volume produced = 75,321 cubic feet

100-year peak flows: $Q_{100} = 104.63$ CFS

100-year volume produced = 113,509 cubic feet

3. All post-development flows will be less than pre-development flows leaving the site in both intensity and volume. There will be no need to provide any mitigation for increase onsite.

	Pre-development	Post-development	% Decreased
10-year flows	54.66 cfs	52.21 cfs	4.5
10-year volume	55,488 cubic feet	54,533 cubic feet	1.7
25-year flows	72.48 cfs	69.43 cfs	4.2
25-year volume	73.933 cubic feet	73,226 cubic feet	0.9
100-year flows	108.77 cfs	104.63 cfs	3.8
100-year volume	111,747 cubic feet	111,414 cubic feet	0.3

Conclusion

There will be no increase in post-development flows leaving the site from pre-development conditions. All onsite flows will continue as they have historically from the east portion of the site to Opal Avenue to the west.

Prepared By:

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APPENDIX

Figure 1.1	PRE-DEVELOPMENT FLOW CALCULATIONS – 10-YEAR STORM
Figure 1.2	PRE-DEVELOPMENT FLOW CALCULATIONS – 25-YEAR STORM
Figure 1.3	PRE-DEVELOPMENT FLOW CALCULATIONS – 100-YEAR STORM
Figure 2.1	POST DEVELOPMENT FLOW CALCULATIONS – 10-YEAR STORM
Figure 2.2	POST DEVELOPMENT FLOW CALCULATIONS – 25-YEAR STORM
Figure 2.3	POST DEVELOPMENT FLOW CALCULATIONS – 100-YEAR STORM
Figure 3.1	ISOHYETAL MAP
Figure 3.2	SOIL GROUP
Figure 4.1	PRE-DEVELOPMENT TRIBUTARY MAP
Figure 4.2	POST-DEVELOPMENT TRIBUTARY MAP

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 SAN BERNARDINO CO. HYDROLOGY CRITERION)
(c) Copyright 1983-2015 Advanced Engineering Software (aes)
Ver. 22.0 Release Date: 07/01/2015 License ID 1533

Analysis prepared by:

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***** DESCRIPTION OF STUDY *****
* 154801 - APN 0298-063-07 *
* PRE-DEVELOPMENT DRAINAGE STUDY *
* 10-YEAR STORM EVENT *

FILE NAME: 154801PR.DAT
TIME/DATE OF STUDY: 11:53 04/04/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE(INCH) = 4.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL
10-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 0.816
100-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 1.410
COMPUTED RAINFALL INTENSITY DATA:
STORM EVENT = 10.00 1-HOUR INTENSITY(INCH/HOUR) = 0.8242
SLOPE OF INTENSITY DURATION CURVE = 0.6000

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

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*****
FLOW PROCESS FROM NODE      1.00 TO NODE      2.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) =   621.00
ELEVATION DATA: UPSTREAM(FEET) =    84.53  DOWNSTREAM(FEET) =    64.42

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =   12.564
* 10 YEAR RAINFALL INTENSITY(INCH/HR) =   2.106
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS  Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
PUBLIC PARK              A      5.19    0.98    0.850   32  12.56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =   0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =   0.850
SUBAREA RUNOFF(CFS) =      5.97
TOTAL AREA(ACRES) =      5.19  PEAK FLOW RATE(CFS) =      5.97

*****
FLOW PROCESS FROM NODE      2.00 TO NODE      3.00 IS CODE =  82
-----
>>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) =   636.00
ELEVATION DATA: UPSTREAM(FEET) =    64.42  DOWNSTREAM(FEET) =    48.69

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    9.313
* 10 YEAR RAINFALL INTENSITY(INCH/HR) =   2.520
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS  Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
MOBILE HOME PARK         A      2.63    0.98    0.250   32   9.31
CONDOMINIUMS             A      2.63    0.98    0.350   32   9.98
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =   0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =   0.300
SUBAREA AREA(ACRES) =    5.26  INITIAL SUBAREA RUNOFF(CFS) =   10.55

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:
MAINLINE Tc(MIN.) =   12.56
* 10 YEAR RAINFALL INTENSITY(INCH/HR) =   2.106
SUBAREA AREA(ACRES) =    5.26  SUBAREA RUNOFF(CFS) =    8.58
EFFECTIVE AREA(ACRES) =   10.45  AREA-AVERAGED Fm(INCH/HR) =   0.56
AREA-AVERAGED Fp(INCH/HR) =   0.98  AREA-AVERAGED Ap =   0.57
TOTAL AREA(ACRES) =   10.5  PEAK FLOW RATE(CFS) =   14.55

*****
FLOW PROCESS FROM NODE      4.00 TO NODE      5.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

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>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 695.00
ELEVATION DATA: UPSTREAM(FEET) = 84.53 DOWNSTREAM(FEET) = 53.42

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.421
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.120
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
".4 DWELLING/ACRE"      A        7.44    0.98    0.900    32    12.42
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.900
SUBAREA RUNOFF(CFS) = 8.32
TOTAL AREA(ACRES) = 7.44 PEAK FLOW RATE(CFS) = 8.32

*****
FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 82
-----
>>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 925.00
ELEVATION DATA: UPSTREAM(FEET) = 53.42 DOWNSTREAM(FEET) = 45.62

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.138
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.150
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL
A        4.86    0.98    0.100    32    12.14
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 4.86 INITIAL SUBAREA RUNOFF(CFS) = 8.98

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:
MAINLINE Tc(MIN.) = 12.42
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.120
SUBAREA AREA(ACRES) = 4.86 SUBAREA RUNOFF(CFS) = 8.85
EFFECTIVE AREA(ACRES) = 12.30 AREA-AVERAGED Fm(INCH/HR) = 0.57
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.58
TOTAL AREA(ACRES) = 12.3 PEAK FLOW RATE(CFS) = 17.17

*****
FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 679.00
ELEVATION DATA: UPSTREAM(FEET) = 86.62 DOWNSTREAM(FEET) = 52.76

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Figure 1.1
Page 3 of 5

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM $T_c(MIN.) = 12.042$
 * 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.160
 SUBAREA T_c AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
RESIDENTIAL						
".4 DWELLING/ACRE"	A	4.85	0.98	0.900	32	12.04

 SUBAREA AVERAGE PERVIOUS LOSS RATE, $F_p(INCH/HR) = 0.98$
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.900$
 SUBAREA RUNOFF(CFS) = 5.60
 TOTAL AREA(ACRES) = 4.85 PEAK FLOW RATE(CFS) = 5.60

 FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 82

>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE T_c ,<<<<<
 >>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) =	629.00
ELEVATION DATA: UPSTREAM(FEET) =	52.76 DOWNSTREAM(FEET) = 47.51

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM $T_c(MIN.) = 10.424$
 * 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.356
 SUBAREA T_c AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	A	4.84	0.98	0.100	32	10.42

 SUBAREA AVERAGE PERVIOUS LOSS RATE, $F_p(INCH/HR) = 0.98$
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.100$
 SUBAREA AREA(ACRES) = 4.84 INITIAL SUBAREA RUNOFF(CFS) = 9.84

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE T_c :
 MAINLINE $T_c(MIN.) = 12.04$
 * 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.160
 SUBAREA AREA(ACRES) = 4.84 SUBAREA RUNOFF(CFS) = 8.98
 EFFECTIVE AREA(ACRES) = 9.69 AREA-AVERAGED $F_m(INCH/HR) = 0.49$
 AREA-AVERAGED $F_p(INCH/HR) = 0.98$ AREA-AVERAGED $A_p = 0.50$
 TOTAL AREA(ACRES) = 9.7 PEAK FLOW RATE(CFS) = 14.58

 FLOW PROCESS FROM NODE 10.00 TO NODE 11.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) =	217.00
ELEVATION DATA: UPSTREAM(FEET) =	52.40 DOWNSTREAM(FEET) = 46.53

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM $T_c(MIN.) = 5.383$
 * 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.502

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	A	2.73	0.98	0.100	32	5.38

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 8.36

TOTAL AREA(ACRES) = 2.73 PEAK FLOW RATE(CFS) = 8.36

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 2.7 TC(MIN.) = 5.38

EFFECTIVE AREA(ACRES) = 2.73 AREA-AVERAGED Fm(INCH/HR)= 0.10

AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.100

PEAK FLOW RATE(CFS) = 8.36

=====

=====

END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
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Ver. 22.0 Release Date: 07/01/2015 License ID 1533

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***** DESCRIPTION OF STUDY *****
* 154801 - APN 0298-063-07 *
* PRE-DEVELOPMENT DRAINAGE STUDY *
* 25-YEAR STORM EVENT *

FILE NAME: 154801PR.DAT
TIME/DATE OF STUDY: 11:52 04/04/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 25.00
SPECIFIED MINIMUM PIPE SIZE(INCH) = 4.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL
10-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 0.816
100-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 1.410
COMPUTED RAINFALL INTENSITY DATA:
STORM EVENT = 25.00 1-HOUR INTENSITY(INCH/HOUR) = 1.0318
SLOPE OF INTENSITY DURATION CURVE = 0.6000

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED


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*****
FLOW PROCESS FROM NODE      1.00 TO NODE      2.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) =   621.00
ELEVATION DATA: UPSTREAM(FEET) =    84.53  DOWNSTREAM(FEET) =    64.42

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =   12.564
* 25 YEAR RAINFALL INTENSITY(INCH/HR) =   2.636
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
PUBLIC PARK              A      5.19      0.98      0.850      32      12.56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =   0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =   0.850
SUBAREA RUNOFF(CFS) =      8.44
TOTAL AREA(ACRES) =      5.19  PEAK FLOW RATE(CFS) =      8.44

*****
FLOW PROCESS FROM NODE      2.00 TO NODE      3.00 IS CODE =  82
-----
>>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) =   636.00
ELEVATION DATA: UPSTREAM(FEET) =    64.42  DOWNSTREAM(FEET) =    48.69

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    9.313
* 25 YEAR RAINFALL INTENSITY(INCH/HR) =   3.155
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
MOBILE HOME PARK         A      2.63      0.98      0.250      32      9.31
CONDOMINIUMS             A      2.63      0.98      0.350      32      9.98
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =   0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =   0.300
SUBAREA AREA(ACRES) =    5.26  INITIAL SUBAREA RUNOFF(CFS) =   13.55

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:
MAINLINE Tc(MIN.) =   12.56
* 25 YEAR RAINFALL INTENSITY(INCH/HR) =   2.636
SUBAREA AREA(ACRES) =    5.26  SUBAREA RUNOFF(CFS) =   11.10
EFFECTIVE AREA(ACRES) =   10.45  AREA-AVERAGED Fm(INCH/HR) =   0.56
AREA-AVERAGED Fp(INCH/HR) =   0.98  AREA-AVERAGED Ap =   0.57
TOTAL AREA(ACRES) =   10.5  PEAK FLOW RATE(CFS) =   19.54

*****
FLOW PROCESS FROM NODE      4.00 TO NODE      5.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

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>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH( FEET) =    695.00
ELEVATION DATA: UPSTREAM( FEET) =    84.53  DOWNSTREAM( FEET) =    53.42

Tc = K*[ (LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    12.421
* 25 YEAR RAINFALL INTENSITY(INCH/HR) =    2.655
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
".4 DWELLING/ACRE"      A        7.44      0.98      0.900      32      12.42
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =    0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =    0.900
SUBAREA RUNOFF(CFS) =    11.90
TOTAL AREA(ACRES) =    7.44  PEAK FLOW RATE(CFS) =    11.90

*****
FLOW PROCESS FROM NODE    5.00 TO NODE    6.00 IS CODE =    82
-----
>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<
=====
INITIAL SUBAREA FLOW-LENGTH( FEET) =    925.00
ELEVATION DATA: UPSTREAM( FEET) =    53.42  DOWNSTREAM( FEET) =    45.62

Tc = K*[ (LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    12.138
* 25 YEAR RAINFALL INTENSITY(INCH/HR) =    2.692
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL
A        4.86      0.98      0.100      32      12.14
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =    0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =    0.100
SUBAREA AREA(ACRES) =    4.86  INITIAL SUBAREA RUNOFF(CFS) =    11.35

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:
MAINLINE Tc(MIN.) =    12.42
* 25 YEAR RAINFALL INTENSITY(INCH/HR) =    2.655
SUBAREA AREA(ACRES) =    4.86  SUBAREA RUNOFF(CFS) =    11.18
EFFECTIVE AREA(ACRES) =    12.30  AREA-AVERAGED Fm(INCH/HR) =    0.57
AREA-AVERAGED Fp(INCH/HR) =    0.98  AREA-AVERAGED Ap =    0.58
TOTAL AREA(ACRES) =    12.3  PEAK FLOW RATE(CFS) =    23.08

*****
FLOW PROCESS FROM NODE    7.00 TO NODE    8.00 IS CODE =    21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH( FEET) =    679.00
ELEVATION DATA: UPSTREAM( FEET) =    86.62  DOWNSTREAM( FEET) =    52.76

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$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM $T_c(MIN.) = 12.042$
 * 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.704
 SUBAREA T_c AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
RESIDENTIAL						
" .4 DWELLING/ACRE"	A	4.85	0.98	0.900	32	12.04

 SUBAREA AVERAGE PERVIOUS LOSS RATE, $F_p(INCH/HR) = 0.98$
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.900$
 SUBAREA RUNOFF(CFS) = 7.97
 TOTAL AREA(ACRES) = 4.85 PEAK FLOW RATE(CFS) = 7.97

 FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 82

>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE T_c ,<<<<<
 >>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 629.00
 ELEVATION DATA: UPSTREAM(FEET) = 52.76 DOWNSTREAM(FEET) = 47.51

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM $T_c(MIN.) = 10.424$
 * 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.949
 SUBAREA T_c AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	A	4.84	0.98	0.100	32	10.42

 SUBAREA AVERAGE PERVIOUS LOSS RATE, $F_p(INCH/HR) = 0.98$
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, $A_p = 0.100$
 SUBAREA AREA(ACRES) = 4.84 INITIAL SUBAREA RUNOFF(CFS) = 12.42

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE T_c :
 MAINLINE $T_c(MIN.) = 12.04$
 * 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.704
 SUBAREA AREA(ACRES) = 4.84 SUBAREA RUNOFF(CFS) = 11.36
 EFFECTIVE AREA(ACRES) = 9.69 AREA-AVERAGED $F_m(INCH/HR) = 0.49$
 AREA-AVERAGED $F_p(INCH/HR) = 0.98$ AREA-AVERAGED $A_p = 0.50$
 TOTAL AREA(ACRES) = 9.7 PEAK FLOW RATE(CFS) = 19.33

 FLOW PROCESS FROM NODE 10.00 TO NODE 11.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 217.00
 ELEVATION DATA: UPSTREAM(FEET) = 52.40 DOWNSTREAM(FEET) = 46.53

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM $T_c(MIN.) = 5.383$
 * 25 YEAR RAINFALL INTENSITY(INCH/HR) = 4.384

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	A	2.73	0.98	0.100	32	5.38

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 10.53

TOTAL AREA(ACRES) = 2.73 PEAK FLOW RATE(CFS) = 10.53

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 2.7 TC(MIN.) = 5.38

EFFECTIVE AREA(ACRES) = 2.73 AREA-AVERAGED Fm(INCH/HR)= 0.10

AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.100

PEAK FLOW RATE(CFS) = 10.53

=====

END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
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***** DESCRIPTION OF STUDY *****
* 154801 - APN 0298-063-07 *
* PRE-DEVELOPMENT DRAINAGE STUDY *
* 100-YEAR STORM EVENT *

FILE NAME: 154801PR.DAT
TIME/DATE OF STUDY: 11:52 04/04/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

---*TIME-OF-CONCENTRATION MODEL*---

USER SPECIFIED STORM EVENT(YEAR) = 100.00
SPECIFIED MINIMUM PIPE SIZE(INCH) = 4.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL
10-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 0.816
100-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 1.410
COMPUTED RAINFALL INTENSITY DATA:
STORM EVENT = 100.00 1-HOUR INTENSITY(INCH/HOUR) = 1.4100
SLOPE OF INTENSITY DURATION CURVE = 0.6000

ANTECEDENT MOISTURE CONDITION (AMC) III ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP HIKE (FT) (FT) (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00 0.0313 0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

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*****
FLOW PROCESS FROM NODE      1.00 TO NODE      2.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 621.00
ELEVATION DATA: UPSTREAM(FEET) = 84.53 DOWNSTREAM(FEET) = 64.42

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.564
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.603
SUBAREA Tc AND LOSS RATE DATA(AMC III):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
PUBLIC PARK              A      5.19      0.74      0.850      52      12.56
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850
SUBAREA RUNOFF(CFS) = 13.88
TOTAL AREA(ACRES) = 5.19 PEAK FLOW RATE(CFS) = 13.88

*****
FLOW PROCESS FROM NODE      2.00 TO NODE      3.00 IS CODE = 82
-----
>>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 636.00
ELEVATION DATA: UPSTREAM(FEET) = 64.42 DOWNSTREAM(FEET) = 48.69

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.313
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.312
SUBAREA Tc AND LOSS RATE DATA(AMC III):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
MOBILE HOME PARK          A      2.63      0.74      0.250      52      9.31
CONDOMINIUMS              A      2.63      0.74      0.350      52      9.98
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.300
SUBAREA AREA(ACRES) = 5.26 INITIAL SUBAREA RUNOFF(CFS) = 19.36

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:
MAINLINE Tc(MIN.) = 12.56
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.603
SUBAREA AREA(ACRES) = 5.26 SUBAREA RUNOFF(CFS) = 16.00
EFFECTIVE AREA(ACRES) = 10.45 AREA-AVERAGED Fm(INCH/HR) = 0.43
AREA-AVERAGED Fp(INCH/HR) = 0.74 AREA-AVERAGED Ap = 0.57
TOTAL AREA(ACRES) = 10.5 PEAK FLOW RATE(CFS) = 29.88

*****
FLOW PROCESS FROM NODE      4.00 TO NODE      5.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

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>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 695.00
ELEVATION DATA: UPSTREAM(FEET) = 84.53 DOWNSTREAM(FEET) = 53.42

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.421
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.628
SUBAREA Tc AND LOSS RATE DATA(AMC III):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS  Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
".4 DWELLING/ACRE"      A        7.44    0.74    0.900    52   12.42
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.900
SUBAREA RUNOFF(CFS) = 19.82
TOTAL AREA(ACRES) = 7.44 PEAK FLOW RATE(CFS) = 19.82

*****
FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 82
-----
>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 925.00
ELEVATION DATA: UPSTREAM(FEET) = 53.42 DOWNSTREAM(FEET) = 45.62

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.138
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.678
SUBAREA Tc AND LOSS RATE DATA(AMC III):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS  Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL
A        4.86    0.74    0.100    52   12.14
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 4.86 INITIAL SUBAREA RUNOFF(CFS) = 15.76

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:
MAINLINE Tc(MIN.) = 12.42
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.628
SUBAREA AREA(ACRES) = 4.86 SUBAREA RUNOFF(CFS) = 15.54
EFFECTIVE AREA(ACRES) = 12.30 AREA-AVERAGED Fm(INCH/HR) = 0.43
AREA-AVERAGED Fp(INCH/HR) = 0.74 AREA-AVERAGED Ap = 0.58
TOTAL AREA(ACRES) = 12.3 PEAK FLOW RATE(CFS) = 35.36

*****
FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 679.00
ELEVATION DATA: UPSTREAM(FEET) = 86.62 DOWNSTREAM(FEET) = 52.76

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Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.042
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.696
SUBAREA Tc AND LOSS RATE DATA(AMC III):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
".4 DWELLING/ACRE"      A        4.85      0.74      0.900      52      12.04
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.900
SUBAREA RUNOFF(CFS) = 13.22
TOTAL AREA(ACRES) = 4.85 PEAK FLOW RATE(CFS) = 13.22

*****
FLOW PROCESS FROM NODE      8.00 TO NODE      9.00 IS CODE = 82
-----
>>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 629.00
ELEVATION DATA: UPSTREAM(FEET) = 52.76 DOWNSTREAM(FEET) = 47.51

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.424
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.030
SUBAREA Tc AND LOSS RATE DATA(AMC III):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL
A        4.84      0.74      0.100      52      10.42
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 4.84 INITIAL SUBAREA RUNOFF(CFS) = 17.23

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:
MAINLINE Tc(MIN.) = 12.04
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.696
SUBAREA AREA(ACRES) = 4.84 SUBAREA RUNOFF(CFS) = 15.77
EFFECTIVE AREA(ACRES) = 9.69 AREA-AVERAGED Fm(INCH/HR) = 0.37
AREA-AVERAGED Fp(INCH/HR) = 0.74 AREA-AVERAGED Ap = 0.50
TOTAL AREA(ACRES) = 9.7 PEAK FLOW RATE(CFS) = 28.99

*****
FLOW PROCESS FROM NODE      10.00 TO NODE      11.00 IS CODE = 21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 217.00
ELEVATION DATA: UPSTREAM(FEET) = 52.40 DOWNSTREAM(FEET) = 46.53

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 5.383
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.991

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SUBAREA Tc AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	A	2.73	0.74	0.100	52	5.38

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 14.54

TOTAL AREA(ACRES) = 2.73 PEAK FLOW RATE(CFS) = 14.54

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 2.7 TC(MIN.) = 5.38

EFFECTIVE AREA(ACRES) = 2.73 AREA-AVERAGED Fm(INCH/HR)= 0.07

AREA-AVERAGED Fp(INCH/HR) = 0.74 AREA-AVERAGED Ap = 0.100

PEAK FLOW RATE(CFS) = 14.54

=====

END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 SAN BERNARDINO CO. HYDROLOGY CRITERION)
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Ver. 22.0 Release Date: 07/01/2015 License ID 1533

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***** DESCRIPTION OF STUDY *****
* 154801 - APN 0298-063-07 *
* POST-DEVELOPMENT DRAINAGE STUDY *
* 10-YEAR STORM EVENT *

FILE NAME: 154801PO.DAT
TIME/DATE OF STUDY: 13:27 04/04/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 10.00
SPECIFIED MINIMUM PIPE SIZE(INCH) = 4.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL
10-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 0.816
100-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 1.410
COMPUTED RAINFALL INTENSITY DATA:
STORM EVENT = 10.00 1-HOUR INTENSITY(INCH/HOUR) = 0.8242
SLOPE OF INTENSITY DURATION CURVE = 0.6000

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

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*****
FLOW PROCESS FROM NODE      1.00 TO NODE      2.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH( FEET) =    621.00
ELEVATION DATA: UPSTREAM( FEET) =    84.53  DOWNSTREAM( FEET) =    64.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =   12.149
* 10 YEAR RAINFALL INTENSITY(INCH/HR) =   2.149
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
"1 DWELLING/ACRE"      A         5.19     0.98     0.800     32     12.15
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.800
SUBAREA RUNOFF(CFS) =      6.39
TOTAL AREA(ACRES) =      5.19  PEAK FLOW RATE(CFS) =      6.39

*****
FLOW PROCESS FROM NODE      2.00 TO NODE      3.00 IS CODE =  82
-----
>>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<
=====
INITIAL SUBAREA FLOW-LENGTH( FEET) =    710.00
ELEVATION DATA: UPSTREAM( FEET) =    64.00  DOWNSTREAM( FEET) =    48.10

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =   12.172
* 10 YEAR RAINFALL INTENSITY(INCH/HR) =   2.146
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
"3-4 DWELLINGS/ACRE"  A         5.26     0.98     0.600     32     12.17
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =  0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =  0.600
SUBAREA AREA(ACRES) =    5.26  INITIAL SUBAREA RUNOFF(CFS) =    7.39

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:
MAINLINE Tc(MIN.) =   12.15
* 10 YEAR RAINFALL INTENSITY(INCH/HR) =   2.149
SUBAREA AREA(ACRES) =    5.26  SUBAREA RUNOFF(CFS) =    7.40
EFFECTIVE AREA(ACRES) =   10.45  AREA-AVERAGED Fm(INCH/HR) =  0.68
AREA-AVERAGED Fp(INCH/HR) =  0.98  AREA-AVERAGED Ap =  0.70
TOTAL AREA(ACRES) =   10.5  PEAK FLOW RATE(CFS) =   13.80

*****
FLOW PROCESS FROM NODE      4.00 TO NODE      5.00 IS CODE =  21
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>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 657.00
ELEVATION DATA: UPSTREAM(FEET) = 84.53 DOWNSTREAM(FEET) = 65.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.693
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.093
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE              GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
"1 DWELLING/ACRE"          A       7.44      0.98      0.800     32     12.69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.800
SUBAREA RUNOFF(CFS) = 8.79
TOTAL AREA(ACRES) = 7.44 PEAK FLOW RATE(CFS) = 8.79

*****
FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 82
-----
>>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 893.00
ELEVATION DATA: UPSTREAM(FEET) = 65.00 DOWNSTREAM(FEET) = 45.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.844
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.438
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE              GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL
A       4.86      0.98      0.100     32     9.84
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 4.86 INITIAL SUBAREA RUNOFF(CFS) = 10.24

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:
MAINLINE Tc(MIN.) = 12.69
* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.093
SUBAREA AREA(ACRES) = 4.86 SUBAREA RUNOFF(CFS) = 8.73
EFFECTIVE AREA(ACRES) = 12.30 AREA-AVERAGED Fm(INCH/HR) = 0.51
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.52
TOTAL AREA(ACRES) = 12.3 PEAK FLOW RATE(CFS) = 17.52

*****
FLOW PROCESS FROM NODE 6.00 TO NODE 6.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

```

TIME OF CONCENTRATION(MIN.) = 12.69
 RAINFALL INTENSITY(INCH/HR) = 2.09
 AREA-AVERAGED Fm(INCH/HR) = 0.51
 AREA-AVERAGED Fp(INCH/HR) = 0.98
 AREA-AVERAGED Ap = 0.52
 EFFECTIVE STREAM AREA(ACRES) = 12.30
 TOTAL STREAM AREA(ACRES) = 12.30
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 17.52

 FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 619.00
 ELEVATION DATA: UPSTREAM(FEET) = 85.20 DOWNSTREAM(FEET) = 52.76

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.066

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.273

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
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RESIDENTIAL

"1 DWELLING/ACRE"	A	4.85	0.98	0.800	32	11.07
-------------------	---	------	------	-------	----	-------

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.800

SUBAREA RUNOFF(CFS) = 6.52

TOTAL AREA(ACRES) = 4.85 PEAK FLOW RATE(CFS) = 6.52

 FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 82

>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<
 >>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<

=====
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 629.00
 ELEVATION DATA: UPSTREAM(FEET) = 52.76 DOWNSTREAM(FEET) = 47.51

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.424

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.356

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
-------------------------------	-------------------	-----------------	-----------------	-----------------	-----------	--------------

COMMERCIAL

	A	4.84	0.98	0.100	32	10.42
--	---	------	------	-------	----	-------

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA AREA(ACRES) = 4.84 INITIAL SUBAREA RUNOFF(CFS) = 9.84

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:

MAINLINE Tc(MIN.) = 11.07

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 2.273

SUBAREA AREA(ACRES) = 4.84 SUBAREA RUNOFF(CFS) = 9.47
 EFFECTIVE AREA(ACRES) = 9.69 AREA-AVERAGED Fm(INCH/HR) = 0.44
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.45
 TOTAL AREA(ACRES) = 9.7 PEAK FLOW RATE(CFS) = 15.99

FLOW PROCESS FROM NODE 10.00 TO NODE 6.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 349.00

ELEVATION DATA: UPSTREAM(FEET) = 52.40 DOWNSTREAM(FEET) = 45.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 6.835

* 10 YEAR RAINFALL INTENSITY(INCH/HR) = 3.034

SUBAREA T_c AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	A	2.73	0.98	0.100	32	6.83

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 7.22

TOTAL AREA(ACRES) = 2.73 PEAK FLOW RATE(CFS) = 7.22

FLOW PROCESS FROM NODE 6.00 TO NODE 6.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 6.83

RAINFALL INTENSITY(INCH/HR) = 3.03

AREA-AVERAGED Fm(INCH/HR) = 0.10

AREA-AVERAGED Fp(INCH/HR) = 0.98

AREA-AVERAGED Ap = 0.10

EFFECTIVE STREAM AREA(ACRES) = 2.73

TOTAL STREAM AREA(ACRES) = 2.73

PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.22

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	17.52	12.69	2.093	0.98(0.51)	0.52	12.3	4.00
2	7.22	6.83	3.034	0.98(0.10)	0.10	2.7	10.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM	Q	T_c	Intensity	Fp(Fm)	Ap	Ae	HEADWATER
--------	---	-------	-----------	--------	----	----	-----------

NUMBER	(CFS)	(MIN.)	(INCH/HR)	(INCH/HR)		(ACRES)	NODE
1	22.26	6.83	3.034	0.98 (0.39)	0.40	9.4	10.00
2	22.42	12.69	2.093	0.97 (0.44)	0.45	15.0	4.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 22.42 Tc(MIN.) = 12.69
EFFECTIVE AREA(ACRES) = 15.03 AREA-AVERAGED Fm(INCH/HR) = 0.44
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.45
TOTAL AREA(ACRES) = 15.0
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 6.00 = 657.00 FEET.

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 15.0 TC(MIN.) = 12.69
EFFECTIVE AREA(ACRES) = 15.03 AREA-AVERAGED Fm(INCH/HR) = 0.44
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.447
PEAK FLOW RATE(CFS) = 22.42

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	22.26	6.83	3.034	0.98 (0.39)	0.40	9.4	10.00
2	22.42	12.69	2.093	0.97 (0.44)	0.45	15.0	4.00

END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 SAN BERNARDINO CO. HYDROLOGY CRITERION)
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Ver. 22.0 Release Date: 07/01/2015 License ID 1533

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***** DESCRIPTION OF STUDY *****
* 154801 - APN 0298-063-07 *
* POST-DEVELOPMENT DRAINAGE STUDY *
* 25-YEAR STORM EVENT *

FILE NAME: 154801PO.DAT
TIME/DATE OF STUDY: 13:26 04/04/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 25.00
SPECIFIED MINIMUM PIPE SIZE(INCH) = 4.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL
10-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 0.816
100-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 1.410
COMPUTED RAINFALL INTENSITY DATA:
STORM EVENT = 25.00 1-HOUR INTENSITY(INCH/HOUR) = 1.0318
SLOPE OF INTENSITY DURATION CURVE = 0.6000

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED


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*****
FLOW PROCESS FROM NODE      1.00 TO NODE      2.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) =   621.00
ELEVATION DATA: UPSTREAM(FEET) =    84.53  DOWNSTREAM(FEET) =    64.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =   12.149
* 25 YEAR RAINFALL INTENSITY(INCH/HR) =   2.690
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE           GROUP  (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
RESIDENTIAL
"1 DWELLING/ACRE"      A         5.19      0.98      0.800      32   12.15
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =   0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =   0.800
SUBAREA RUNOFF(CFS) =      8.92
TOTAL AREA(ACRES) =      5.19  PEAK FLOW RATE(CFS) =      8.92

*****
FLOW PROCESS FROM NODE      2.00 TO NODE      3.00 IS CODE =  82
-----
>>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) =   710.00
ELEVATION DATA: UPSTREAM(FEET) =    64.00  DOWNSTREAM(FEET) =    48.10

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =   12.172
* 25 YEAR RAINFALL INTENSITY(INCH/HR) =   2.687
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS      Tc
    LAND USE           GROUP  (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
RESIDENTIAL
"3-4 DWELLINGS/ACRE"  A         5.26      0.98      0.600      32   12.17
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =   0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =   0.600
SUBAREA AREA(ACRES) =      5.26  INITIAL SUBAREA RUNOFF(CFS) =      9.95

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:
MAINLINE Tc(MIN.) =   12.15
* 25 YEAR RAINFALL INTENSITY(INCH/HR) =   2.690
SUBAREA AREA(ACRES) =      5.26  SUBAREA RUNOFF(CFS) =      9.97
EFFECTIVE AREA(ACRES) =     10.45  AREA-AVERAGED Fm(INCH/HR) =   0.68
AREA-AVERAGED Fp(INCH/HR) =   0.98  AREA-AVERAGED Ap =   0.70
TOTAL AREA(ACRES) =     10.5  PEAK FLOW RATE(CFS) =     18.89

*****
FLOW PROCESS FROM NODE      4.00 TO NODE      5.00 IS CODE =  21
-----

```

```

>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 657.00
ELEVATION DATA: UPSTREAM(FEET) = 84.53 DOWNSTREAM(FEET) = 65.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.693
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.620
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/    SCS SOIL    AREA    Fp    Ap    SCS    Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
"1 DWELLING/ACRE"      A        7.44    0.98    0.800    32    12.69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.800
SUBAREA RUNOFF(CFS) = 12.32
TOTAL AREA(ACRES) = 7.44 PEAK FLOW RATE(CFS) = 12.32

*****
FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 82
=====
>>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 893.00
ELEVATION DATA: UPSTREAM(FEET) = 65.00 DOWNSTREAM(FEET) = 45.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.844
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.052
SUBAREA Tc AND LOSS RATE DATA(AMC II):
  DEVELOPMENT TYPE/    SCS SOIL    AREA    Fp    Ap    SCS    Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL
A        4.86    0.98    0.100    32    9.84
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 4.86 INITIAL SUBAREA RUNOFF(CFS) = 12.92

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:
MAINLINE Tc(MIN.) = 12.69
* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.620
SUBAREA AREA(ACRES) = 4.86 SUBAREA RUNOFF(CFS) = 11.03
EFFECTIVE AREA(ACRES) = 12.30 AREA-AVERAGED Fm(INCH/HR) = 0.51
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.52
TOTAL AREA(ACRES) = 12.3 PEAK FLOW RATE(CFS) = 23.36

*****
FLOW PROCESS FROM NODE 6.00 TO NODE 6.00 IS CODE = 1
=====
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

```

TIME OF CONCENTRATION(MIN.) = 12.69
 RAINFALL INTENSITY(INCH/HR) = 2.62
 AREA-AVERAGED Fm(INCH/HR) = 0.51
 AREA-AVERAGED Fp(INCH/HR) = 0.98
 AREA-AVERAGED Ap = 0.52
 EFFECTIVE STREAM AREA(ACRES) = 12.30
 TOTAL STREAM AREA(ACRES) = 12.30
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 23.36

 FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 21

>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 619.00
 ELEVATION DATA: UPSTREAM(FEET) = 85.20 DOWNSTREAM(FEET) = 52.76

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 11.066

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.845

SUBAREA T_c AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	T_c (MIN.)
-------------------------------	-------------------	-----------------	-----------------	-----------------	-----------	-----------------

RESIDENTIAL

"1 DWELLING/ACRE"	A	4.85	0.98	0.800	32	11.07
-------------------	---	------	------	-------	----	-------

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.800

SUBAREA RUNOFF(CFS) = 9.01

TOTAL AREA(ACRES) = 4.85 PEAK FLOW RATE(CFS) = 9.01

 FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 82

>>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE T_c ,<<<<<
 >>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 629.00
 ELEVATION DATA: UPSTREAM(FEET) = 52.76 DOWNSTREAM(FEET) = 47.51

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 10.424

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.949

SUBAREA T_c AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	T_c (MIN.)
-------------------------------	-------------------	-----------------	-----------------	-----------------	-----------	-----------------

COMMERCIAL	A	4.84	0.98	0.100	32	10.42
------------	---	------	------	-------	----	-------

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA AREA(ACRES) = 4.84 INITIAL SUBAREA RUNOFF(CFS) = 12.42

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE T_c :

MAINLINE T_c (MIN.) = 11.07

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 2.845

SUBAREA AREA(ACRES) = 4.84 SUBAREA RUNOFF(CFS) = 11.97
 EFFECTIVE AREA(ACRES) = 9.69 AREA-AVERAGED Fm(INCH/HR) = 0.44
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.45
 TOTAL AREA(ACRES) = 9.7 PEAK FLOW RATE(CFS) = 20.98

FLOW PROCESS FROM NODE 10.00 TO NODE 6.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 349.00
 ELEVATION DATA: UPSTREAM(FEET) = 52.40 DOWNSTREAM(FEET) = 45.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 6.835

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.799

SUBAREA T_c AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	A	2.73	0.98	0.100	32	6.83

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 9.09

TOTAL AREA(ACRES) = 2.73 PEAK FLOW RATE(CFS) = 9.09

FLOW PROCESS FROM NODE 6.00 TO NODE 6.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 6.83
 RAINFALL INTENSITY(INCH/HR) = 3.80
 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 2.73
 TOTAL STREAM AREA(ACRES) = 2.73
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 9.09

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	23.36	12.69	2.620	0.98 (0.51)	0.52	12.3	4.00
2	9.09	6.83	3.799	0.98 (0.10)	0.10	2.7	10.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM	Q	T_c	Intensity	Fp(Fm)	Ap	Ae	HEADWATER
--------	---	-------	-----------	--------	----	----	-----------

NUMBER	(CFS)	(MIN.)	(INCH/HR)	(INCH/HR)		(ACRES)	NODE
1	28.70	6.83	3.799	0.98 (0.39)	0.40	9.4	10.00
2	29.56	12.69	2.620	0.97 (0.44)	0.45	15.0	4.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 29.56 TC(MIN.) = 12.69
EFFECTIVE AREA(ACRES) = 15.03 AREA-AVERAGED Fm(INCH/HR) = 0.44
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.45
TOTAL AREA(ACRES) = 15.0
LONGEST FLOWPATH FROM NODE 4.00 TO NODE 6.00 = 657.00 FEET.

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 15.0 TC(MIN.) = 12.69
EFFECTIVE AREA(ACRES) = 15.03 AREA-AVERAGED Fm(INCH/HR)= 0.44
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.447
PEAK FLOW RATE(CFS) = 29.56

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	28.70	6.83	3.799	0.98 (0.39)	0.40	9.4	10.00
2	29.56	12.69	2.620	0.97 (0.44)	0.45	15.0	4.00

=====

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END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 SAN BERNARDINO CO. HYDROLOGY CRITERION)
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Ver. 22.0 Release Date: 07/01/2015 License ID 1533

Analysis prepared by:

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***** DESCRIPTION OF STUDY *****
* 154801 - APN 0298-063-07 *
* POST-DEVELOPMENT DRAINAGE STUDY *
* 100-YEAR STORM EVENT *

FILE NAME: 154801PO.DAT
TIME/DATE OF STUDY: 13:26 04/04/2017

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00
SPECIFIED MINIMUM PIPE SIZE(INCH) = 4.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL
10-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 0.816
100-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 1.410
COMPUTED RAINFALL INTENSITY DATA:
STORM EVENT = 100.00 1-HOUR INTENSITY(INCH/HOUR) = 1.4100
SLOPE OF INTENSITY DURATION CURVE = 0.6000

ANTECEDENT MOISTURE CONDITION (AMC) III ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

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*****
FLOW PROCESS FROM NODE      1.00 TO NODE      2.00 IS CODE =  21
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) =    621.00
ELEVATION DATA: UPSTREAM(FEET) =    84.53  DOWNSTREAM(FEET) =    64.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    12.149
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =    3.676
SUBAREA Tc AND LOSS RATE DATA(AMC III):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS  Tc
    LAND USE          GROUP  (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
RESIDENTIAL
"1 DWELLING/ACRE"      A        5.19    0.74    0.800    52   12.15
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =    0.74
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =    0.800
SUBAREA RUNOFF(CFS) =    14.40
TOTAL AREA(ACRES) =    5.19  PEAK FLOW RATE(CFS) =    14.40

*****
FLOW PROCESS FROM NODE      2.00 TO NODE      3.00 IS CODE =  82
-----
>>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) =    710.00
ELEVATION DATA: UPSTREAM(FEET) =    64.00  DOWNSTREAM(FEET) =    48.10

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) =    12.172
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =    3.672
SUBAREA Tc AND LOSS RATE DATA(AMC III):
  DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS  Tc
    LAND USE          GROUP  (ACRES)  (INCH/HR)  (DECIMAL)  CN  (MIN.)
RESIDENTIAL
"3-4 DWELLINGS/ACRE"  A        5.26    0.74    0.600    52   12.17
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) =    0.74
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap =    0.600
SUBAREA AREA(ACRES) =    5.26  INITIAL SUBAREA RUNOFF(CFS) =    15.28

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:
MAINLINE Tc(MIN.) =    12.15
* 100 YEAR RAINFALL INTENSITY(INCH/HR) =    3.676
SUBAREA AREA(ACRES) =    5.26  SUBAREA RUNOFF(CFS) =    15.29
EFFECTIVE AREA(ACRES) =    10.45  AREA-AVERAGED Fm(INCH/HR) =    0.52
AREA-AVERAGED Fp(INCH/HR) =    0.74  AREA-AVERAGED Ap =    0.70
TOTAL AREA(ACRES) =    10.5  PEAK FLOW RATE(CFS) =    29.69

*****
FLOW PROCESS FROM NODE      4.00 TO NODE      5.00 IS CODE =  21
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>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 657.00
ELEVATION DATA: UPSTREAM(FEET) = 84.53 DOWNSTREAM(FEET) = 65.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.693
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.581
SUBAREA Tc AND LOSS RATE DATA(AMC III):
  DEVELOPMENT TYPE/    SCS SOIL    AREA    Fp    Ap    SCS    Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
"1 DWELLING/ACRE"      A        7.44    0.74    0.800    52    12.69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.800
SUBAREA RUNOFF(CFS) = 20.00
TOTAL AREA(ACRES) = 7.44 PEAK FLOW RATE(CFS) = 20.00

*****
FLOW PROCESS FROM NODE 5.00 TO NODE 6.00 IS CODE = 82
-----
>>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE Tc,<<<<<
>>>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 893.00
ELEVATION DATA: UPSTREAM(FEET) = 65.00 DOWNSTREAM(FEET) = 45.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.844
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.171
SUBAREA Tc AND LOSS RATE DATA(AMC III):
  DEVELOPMENT TYPE/    SCS SOIL    AREA    Fp    Ap    SCS    Tc
    LAND USE          GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL
A        4.86    0.74    0.100    52    9.84
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 4.86 INITIAL SUBAREA RUNOFF(CFS) = 17.92

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE Tc:
MAINLINE Tc(MIN.) = 12.69
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.581
SUBAREA AREA(ACRES) = 4.86 SUBAREA RUNOFF(CFS) = 15.34
EFFECTIVE AREA(ACRES) = 12.30 AREA-AVERAGED Fm(INCH/HR) = 0.39
AREA-AVERAGED Fp(INCH/HR) = 0.74 AREA-AVERAGED Ap = 0.52
TOTAL AREA(ACRES) = 12.3 PEAK FLOW RATE(CFS) = 35.34

*****
FLOW PROCESS FROM NODE 6.00 TO NODE 6.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

```


TIME OF CONCENTRATION(MIN.) = 12.69
 RAINFALL INTENSITY(INCH/HR) = 3.58
 AREA-AVERAGED Fm(INCH/HR) = 0.39
 AREA-AVERAGED Fp(INCH/HR) = 0.74
 AREA-AVERAGED Ap = 0.52
 EFFECTIVE STREAM AREA(ACRES) = 12.30
 TOTAL STREAM AREA(ACRES) = 12.30
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 35.34

 FLOW PROCESS FROM NODE 7.00 TO NODE 8.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 619.00
 ELEVATION DATA: UPSTREAM(FEET) = 85.20 DOWNSTREAM(FEET) = 52.76

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 11.066
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.888
 SUBAREA T_c AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	T_c (MIN.)
RESIDENTIAL						
"1 DWELLING/ACRE"	A	4.85	0.74	0.800	52	11.07

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.800
 SUBAREA RUNOFF(CFS) = 14.38
 TOTAL AREA(ACRES) = 4.85 PEAK FLOW RATE(CFS) = 14.38

 FLOW PROCESS FROM NODE 8.00 TO NODE 9.00 IS CODE = 82

>>>>ADD SUBAREA RUNOFF TO MAINLINE, AT MAINLINE T_c ,<<<<<
 >>>>(AND COMPUTE INITIAL SUBAREA RUNOFF)<<<<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 629.00
 ELEVATION DATA: UPSTREAM(FEET) = 52.76 DOWNSTREAM(FEET) = 47.51

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 10.424
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.030
 SUBAREA T_c AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	A	4.84	0.74	0.100	52	10.42

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 4.84 INITIAL SUBAREA RUNOFF(CFS) = 17.23

** ADD SUBAREA RUNOFF TO MAINLINE AT MAINLINE T_c :
 MAINLINE T_c (MIN.) = 11.07
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.888

SUBAREA AREA(ACRES) = 4.84 SUBAREA RUNOFF(CFS) = 16.61
 EFFECTIVE AREA(ACRES) = 9.69 AREA-AVERAGED Fm(INCH/HR) = 0.33
 AREA-AVERAGED Fp(INCH/HR) = 0.74 AREA-AVERAGED Ap = 0.45
 TOTAL AREA(ACRES) = 9.7 PEAK FLOW RATE(CFS) = 30.99

FLOW PROCESS FROM NODE 10.00 TO NODE 6.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 349.00

ELEVATION DATA: UPSTREAM(FEET) = 52.40 DOWNSTREAM(FEET) = 45.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 6.835

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 5.191

SUBAREA T_c AND LOSS RATE DATA(AMC III):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	T_c (MIN.)
COMMERCIAL	A	2.73	0.74	0.100	52	6.83

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.74

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA RUNOFF(CFS) = 12.57

TOTAL AREA(ACRES) = 2.73 PEAK FLOW RATE(CFS) = 12.57

FLOW PROCESS FROM NODE 6.00 TO NODE 6.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 6.83

RAINFALL INTENSITY(INCH/HR) = 5.19

AREA-AVERAGED Fm(INCH/HR) = 0.07

AREA-AVERAGED Fp(INCH/HR) = 0.74

AREA-AVERAGED Ap = 0.10

EFFECTIVE STREAM AREA(ACRES) = 2.73

TOTAL STREAM AREA(ACRES) = 2.73

PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.57

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	T_c (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	35.34	12.69	3.581	0.74(0.39)	0.52	12.3	4.00
2	12.57	6.83	5.191	0.74(0.07)	0.10	2.7	10.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO

CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM	Q	T_c	Intensity	Fp(Fm)	Ap	Ae	HEADWATER
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NUMBER	(CFS)	(MIN.)	(INCH/HR)	(INCH/HR)		(ACRES)	NODE
1	41.20	6.83	5.191	0.74 (0.30)	0.40	9.4	10.00
2	43.95	12.69	3.581	0.74 (0.33)	0.45	15.0	4.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 43.95 TC(MIN.) = 12.69
 EFFECTIVE AREA(ACRES) = 15.03 AREA-AVERAGED Fm(INCH/HR) = 0.33
 AREA-AVERAGED Fp(INCH/HR) = 0.74 AREA-AVERAGED Ap = 0.45
 TOTAL AREA(ACRES) = 15.0
 LONGEST FLOWPATH FROM NODE 4.00 TO NODE 6.00 = 657.00 FEET.

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END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 15.0 TC(MIN.) = 12.69
 EFFECTIVE AREA(ACRES) = 15.03 AREA-AVERAGED Fm(INCH/HR) = 0.33
 AREA-AVERAGED Fp(INCH/HR) = 0.74 AREA-AVERAGED Ap = 0.447
 PEAK FLOW RATE(CFS) = 43.95

** PEAK FLOW RATE TABLE **

STREAM	Q	Tc	Intensity	Fp(Fm)	Ap	Ae	HEADWATER
NUMBER	(CFS)	(MIN.)	(INCH/HR)	(INCH/HR)		(ACRES)	NODE
1	41.20	6.83	5.191	0.74 (0.30)	0.40	9.4	10.00
2	43.95	12.69	3.581	0.74 (0.33)	0.45	15.0	4.00

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END OF RATIONAL METHOD ANALYSIS



NOAA Atlas 14, Volume 6, Version 2
 Location name: Mentone, California, US*
 Coordinates: 34.0648, -117.1323
 Elevation: 1666 ft*
 * source: Google Maps



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin,
 Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao,
 Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aeriels](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.105 (0.087-0.127)	0.139 (0.115-0.169)	0.187 (0.155-0.228)	0.229 (0.188-0.282)	0.291 (0.231-0.370)	0.341 (0.265-0.443)	0.396 (0.300-0.528)	0.456 (0.336-0.625)	0.544 (0.384-0.778)	0.617 (0.420-0.914)
10-min	0.150 (0.125-0.182)	0.199 (0.166-0.242)	0.268 (0.222-0.327)	0.329 (0.270-0.404)	0.416 (0.331-0.530)	0.489 (0.380-0.636)	0.568 (0.430-0.756)	0.654 (0.481-0.896)	0.779 (0.550-1.11)	0.884 (0.602-1.31)
15-min	0.181 (0.151-0.220)	0.241 (0.200-0.293)	0.325 (0.269-0.396)	0.397 (0.327-0.488)	0.504 (0.400-0.641)	0.591 (0.460-0.769)	0.686 (0.520-0.915)	0.790 (0.582-1.08)	0.942 (0.665-1.35)	1.07 (0.729-1.58)
30-min	0.266 (0.221-0.323)	0.353 (0.293-0.429)	0.476 (0.394-0.580)	0.582 (0.479-0.716)	0.738 (0.586-0.939)	0.867 (0.674-1.13)	1.01 (0.762-1.34)	1.16 (0.853-1.59)	1.38 (0.975-1.98)	1.57 (1.07-2.32)
60-min	0.373 (0.310-0.452)	0.495 (0.411-0.601)	0.666 (0.552-0.812)	0.816 (0.670-1.00)	1.03 (0.821-1.31)	1.21 (0.943-1.58)	1.41 (1.07-1.88)	1.62 (1.20-2.23)	1.93 (1.36-2.77)	2.19 (1.50-3.25)
2-hr	0.536 (0.446-0.651)	0.696 (0.579-0.846)	0.919 (0.761-1.12)	1.11 (0.912-1.36)	1.38 (1.10-1.76)	1.61 (1.25-2.09)	1.85 (1.40-2.46)	2.10 (1.55-2.89)	2.48 (1.75-3.54)	2.78 (1.90-4.12)
3-hr	0.657 (0.547-0.798)	0.846 (0.703-1.03)	1.10 (0.916-1.35)	1.33 (1.09-1.63)	1.64 (1.30-2.09)	1.90 (1.47-2.46)	2.17 (1.64-2.89)	2.46 (1.81-3.37)	2.87 (2.03-4.11)	3.21 (2.19-4.75)
6-hr	0.919 (0.765-1.12)	1.17 (0.974-1.42)	1.51 (1.25-1.85)	1.80 (1.48-2.22)	2.21 (1.75-2.81)	2.53 (1.97-3.29)	2.87 (2.18-3.83)	3.23 (2.38-4.43)	3.73 (2.63-5.34)	4.14 (2.82-6.13)
12-hr	1.25 (1.04-1.51)	1.59 (1.32-1.93)	2.05 (1.70-2.50)	2.43 (1.99-2.98)	2.95 (2.35-3.76)	3.36 (2.62-4.37)	3.79 (2.87-5.05)	4.23 (3.12-5.80)	4.84 (3.42-6.93)	5.33 (3.63-7.90)
24-hr	1.68 (1.49-1.93)	2.16 (1.91-2.49)	2.79 (2.46-3.23)	3.31 (2.90-3.86)	4.03 (3.41-4.85)	4.58 (3.80-5.63)	5.14 (4.16-6.47)	5.72 (4.51-7.41)	6.52 (4.93-8.79)	7.14 (5.22-9.95)
2-day	2.04 (1.80-2.35)	2.66 (2.35-3.07)	3.49 (3.08-4.04)	4.17 (3.65-4.87)	5.11 (4.33-6.16)	5.84 (4.85-7.18)	6.59 (5.34-8.30)	7.37 (5.81-9.54)	8.44 (6.39-11.4)	9.28 (6.79-12.9)
3-day	2.18 (1.93-2.51)	2.88 (2.55-3.33)	3.83 (3.38-4.43)	4.61 (4.03-5.37)	5.69 (4.82-6.86)	6.54 (5.43-8.04)	7.42 (6.01-9.34)	8.33 (6.57-10.8)	9.60 (7.27-12.9)	10.6 (7.76-14.8)
4-day	2.35 (2.08-2.70)	3.13 (2.77-3.61)	4.18 (3.69-4.84)	5.06 (4.42-5.90)	6.27 (5.31-7.56)	7.23 (6.00-8.89)	8.22 (6.66-10.4)	9.26 (7.30-12.0)	10.7 (8.10-14.4)	11.9 (8.67-16.5)
7-day	2.70 (2.39-3.11)	3.65 (3.23-4.21)	4.93 (4.35-5.71)	6.00 (5.25-6.99)	7.48 (6.33-9.01)	8.64 (7.17-10.6)	9.85 (7.98-12.4)	11.1 (8.77-14.4)	12.9 (9.75-17.4)	14.3 (10.4-19.9)
10-day	2.92 (2.59-3.37)	3.98 (3.52-4.59)	5.41 (4.77-6.26)	6.60 (5.77-7.69)	8.25 (6.99-9.94)	9.56 (7.93-11.8)	10.9 (8.84-13.7)	12.3 (9.72-16.0)	14.3 (10.8-19.3)	15.9 (11.6-22.1)
20-day	3.64 (3.22-4.19)	5.01 (4.43-5.78)	6.86 (6.05-7.94)	8.41 (7.36-9.80)	10.6 (8.95-12.7)	12.3 (10.2-15.1)	14.0 (11.4-17.7)	15.9 (12.5-20.6)	18.5 (14.0-24.9)	20.5 (15.0-28.6)
30-day	4.33 (3.83-4.99)	5.96 (5.27-6.88)	8.16 (7.20-9.44)	10.0 (8.76-11.7)	12.6 (10.7-15.2)	14.6 (12.1-18.0)	16.7 (13.6-21.1)	19.0 (15.0-24.6)	22.1 (16.7-29.8)	24.5 (18.0-34.2)
45-day	5.23 (4.63-6.03)	7.14 (6.32-8.24)	9.73 (8.58-11.3)	11.9 (10.4-13.9)	14.9 (12.6-18.0)	17.3 (14.4-21.3)	19.8 (16.1-25.0)	22.5 (17.7-29.1)	26.1 (19.8-35.3)	29.1 (21.3-40.5)
60-day	6.21 (5.50-7.15)	8.37 (7.40-9.65)	11.3 (9.96-13.1)	13.7 (12.0-16.0)	17.2 (14.6-20.7)	19.9 (16.5-24.5)	22.8 (18.4-28.7)	25.8 (20.3-33.4)	30.0 (22.7-40.4)	33.3 (24.4-46.4)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

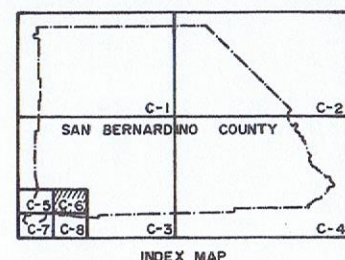
[Back to Top](#)



- LEGEND
- SOIL GROUP BOUNDARY
 - A SOIL GROUP DESIGNATION
 - BOUNDARY OF INDICATED SOURCE

SCALE REDUCED BY 1/2

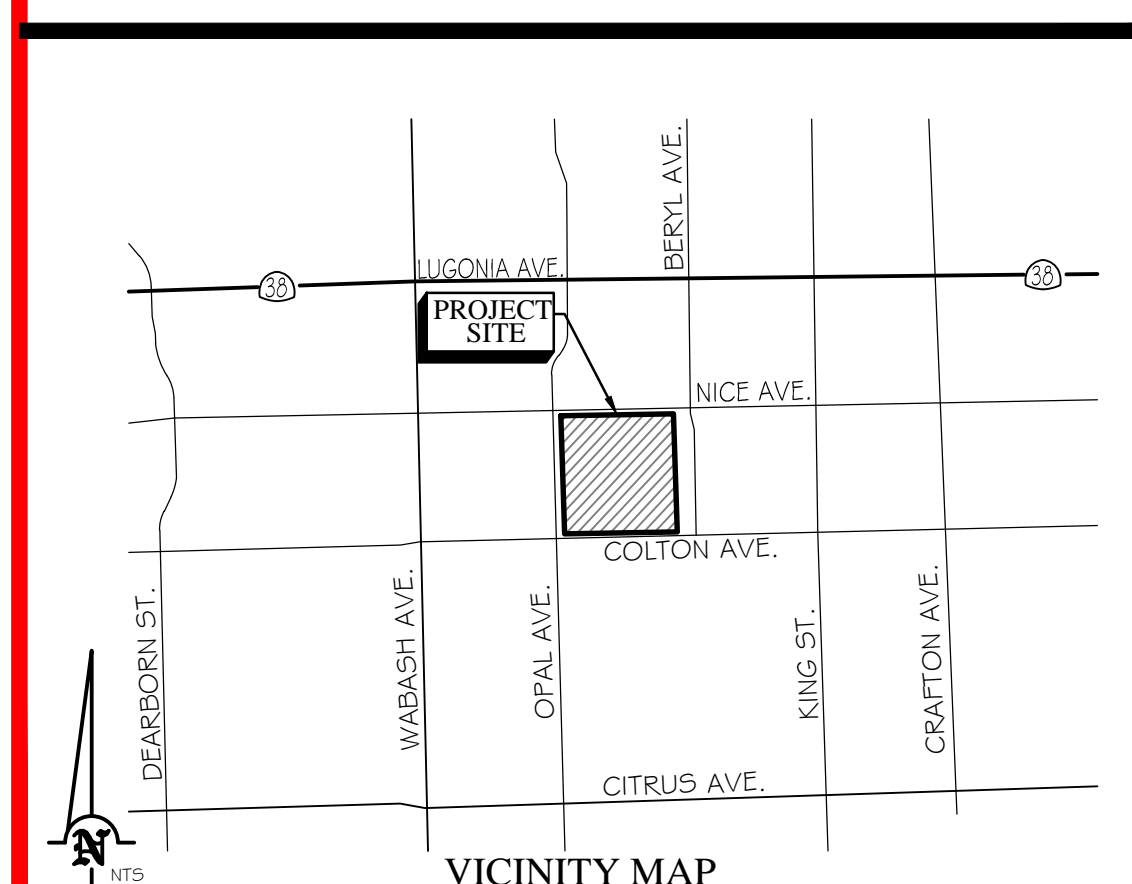
SCALE 1:48,000



SAN BERNARDINO COUNTY
HYDROLOGY MANUAL

HYDROLOGIC SOILS GROUP MAP
FOR
SOUTHWEST-B AREA

801 OPAL AVENUE, COUNTY OF SAN BERNARDINO

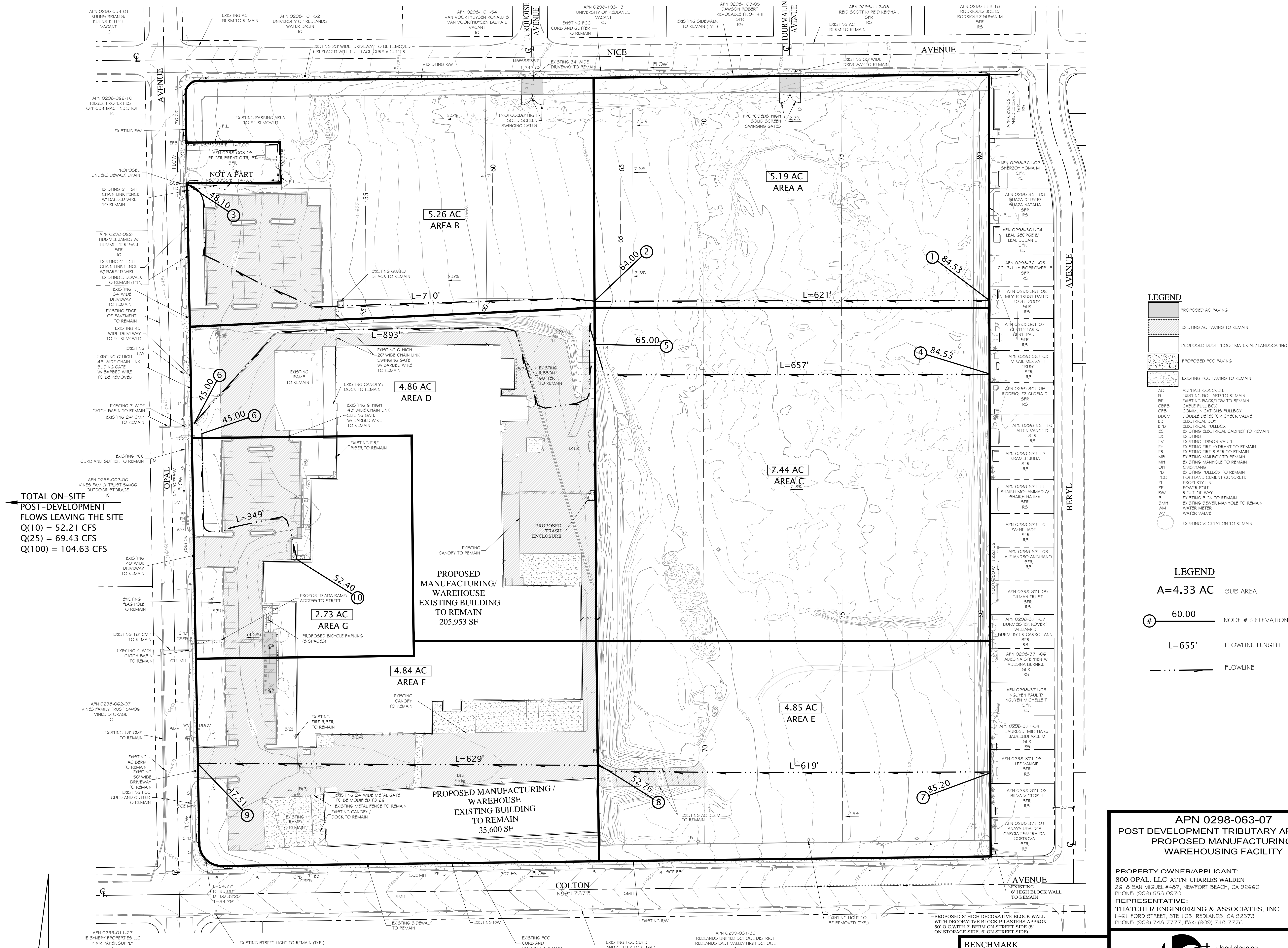


POST DEVELOPMENT TRIBUTARY AREA MAP

PROPOSED MANUFACTURING/ WAREHOUSING FACILITY

APN 0298-063-07

801 OPAL AVENUE, COUNTY OF SAN BERNARDINO



TOTAL ON-SITE
POST-DEVELOPMENT
FLOWS LEAVING THE SITE
Q(10) = 52.21 CFS
Q(25) = 69.43 CFS
Q(100) = 104.63 CFS

- LEGEND**
- PROPOSED AC PAVING
 - EXISTING AC PAVING TO REMAIN
 - PROPOSED DUST PROOF MATERIAL / LANDSCAPING
 - PROPOSED PCC PAVING
 - EXISTING PCC PAVING TO REMAIN
 - AC ASPHALT CONCRETE
 - B EXISTING BOLLARD TO REMAIN
 - CBFB EXISTING BACKFLOW TO REMAIN
 - CPB CABLE PULL BOX
 - DDCV COMMUNICATIONS PULLBOX
 - DC DOUBLE DETECTOR CHECK VALVE
 - EB ELECTRICAL BOX
 - EPB ELECTRICAL PULLBOX
 - EX EXISTING ELECTRICAL CABINET TO REMAIN
 - EV EXISTING EDISON VAULT
 - FR EXISTING FIRE HYDRANT TO REMAIN
 - FR EXISTING FIRE RISER TO REMAIN
 - MB EXISTING MAILBOX TO REMAIN
 - MH EXISTING MANHOLE TO REMAIN
 - OH OVERHANG
 - PS EXISTING PULLBOX TO REMAIN
 - PCC PORTLAND CEMENT CONCRETE
 - PL PROPERTY LINE
 - PP POWER POLE
 - RF RIGHT-OF-WAY
 - S EXISTING SIGN TO REMAIN
 - SMH EXISTING SEWER MANHOLE TO REMAIN
 - WM WATER METER
 - WV WATER VALVE
 - EXISTING VEGETATION TO REMAIN

- LEGEND**
- A=4.33 AC SUB AREA
- 60.00 NODE # ELEVATIONS
- L=655' FLOWLINE LENGTH
- FLOWLINE

SOURCE OF SURVEY
TOPOGRAPHIC SURVEY
DATED AUGUST 2013
CONDUCTED BY
ONPOINT LAND SURVEYING, INC.
1910 ORANGE TREE LANE, SUITE 344
REDLANDS, CALIFORNIA 92374
PHONE: (909) 792-2221

BENCHMARK
CITY OF REDLANDS BENCHMARK R-43:
BRASS DISK IN THE TOP OF CURB @ NE COR. BERYL
AVE. & COLTON AVE. EAST END OF RETURN.
ELEVATION= 1,663.679

BASIS OF BEARING
CENTERLINE OF OPAL AVENUE PER RS 55/28 BEING:
N01°02'59"W.

APN 0298-063-07
POST DEVELOPMENT TRIBUTARY AREA MAP
PROPOSED MANUFACTURING/ WAREHOUSING FACILITY

PROPERTY OWNER/APPLICANT:
800 OPAL, LLC ATTN: CHARLES WALDEN
2616 SAN MIGUEL #467, NEWPORT BEACH, CA 92660
PHONE: (909) 553-0970

REPRESENTATIVE:
THATCHER ENGINEERING & ASSOCIATES, INC
1461 FORD STREET, STE 105, REDLANDS, CA 92373
PHONE: (909) 746-7777, FAX: (909) 746-7776

thatcher engineering & associates, inc.
1461 Ford Street, Suite 105, Redlands, CA 92373

- land planning
- civil engineering
- landscape architecture

Job Number: 154801 Date Prepared: 8/1/17 Drawn By: PF Reference Number: 154801TAM2

