

Drainage Study

For:

George Wanis

9128 Green Rd.

Pinon Hills, CA 92397

Impacting a 3.01 Acre Development

Located On:

NW Corner of Beekley Rd & HWY 138

Pinon Hills CA 92372

San Bernardino County

APN: 3066-191-04

Job No. 2016.0225



PRELIMINARY APPROVED

By Osvaldo Roque at 3:13 pm, Jan 15, 2019

Additional comments can be addressed in the final submittal

***Address comments in final report.**

Purpose:

This report is to provide a drainage study based on the San Bernardino County guidelines for a proposed Commercial Development. The owner of APN 3066-191-04 has applied to build a Fuel Station with Convenience store impacting 1.65 acres of the approximately 3.01 acres parcel. The purpose of this study is to determine whether this site is impacted by off-site drainage flows, and what measures are necessary to protect the development from these flows. Additionally, this study will determine the expected on-site flows produced because of the proposed development.

Site Description

The total property area is approximately 3.01 acres and is located on the NW corner of Beekley Rd. and HWY 138 in the unincorporated community of Pinon Hills, San Bernardino CA. The fully developed site will be composed of directly connected and non-directly connected impervious areas that will drain into an onsite storm water ~~detention or retention system before it leaves the project site.~~ The impervious area will be composed of the structures, driveway, walkways, and parking spaces. ~~Some of this area may drain onto pervious areas such as landscaped planters or bare earth. Runoff from these pervious areas will not enter the storm water retention system until they become saturated.~~

A review of the NOAA 14 Point Precipitation Point Frequency data for the 2-year, 10-year, 25-year and 100-year storms is as follow:

2-year 24hr Storm (inches):	1.71
10-year 24hr Storm (inches):	3.01
25-year 24hr Storm (inches):	3.83
100-year 24hr Storm (inches):	5.10

Already assumed.
See Antecedent
moisture content in
hydrology manual.

... that will capture
the full difference in
post development
runoff volume.

The topographical data that was obtained for the site plan shows that this site slopes to the north at a grade of 6% to 8% with a drop off of the very north east corner of the property down to Beekley Rd.

Research Data:

A review of the Phelan Quad Map, portion attached, finds there is a drainage course that flows near the site although it by-passes the site to the east flowing north east away from the site in a natural drainage facility. There appears to be no impact to the subject project.

A review of FEMA Firm 06071C64500H found the project site was located within a Zone X overlay. (see attached map)

Explain that any other tributary
drainage is intercepted by State
Hwy 138 and does not impact
site.

Use USDA websoils survey,
see attached addendum.

Based on the San Bernardino County Drainage Manual Hydraulic Soils Group map, Figure C-11 the hydrologic soils group for this site is identified as Soil Group C. See attached Figure C-11.

Field Review:

A field review was performed to determine the extent of the drainage area that impacts this site. This review found that the site is not impacted from off-site drainage. The site is relatively flat and slopes to the north with sparse vegetation. The development proposed on the southernmost 1.65 acres of the property remains on the most elevated portion of the property. Current flows within the road right of way appears to remain contained therein due to a dike and or roadway ditch that carry water by the project. Additional improvements from development of this project will enhance protections from potential storm water generated on the roadways. Beekley Rd. is at such a grade that little impact can be expected and likewise improvements on HWY 138 will direct roadway flows away from the project.

Can use this to explain
why off-site drainage is
not analyzed and does
not impact site.

Runoff Calculations:

The following are the results of the pre-developed and post-developed on-site drainage contribution. Results determined based on the San Bernardino County Hydrology Manual. Peak Runoff analysis is a comparison of the 100yr 1hr for pre-and post-conditions using the Rational Method. Results of a Rational Method analysis shows a post developed TC of 15.1 minutes. See attached Calculation Sheet "A".

	2yr24hr Storm (cf)	10yr24hr Storm (cf)	100yr24hr Storm (cf)	Time of Concentration (min)	Peak Runoff (cfs)
Pre	4,250	22,791	61,298	13.1	10.6
Post	6,332	22,107	58,798	15.1	8.7
Increase	1,924	-684	-2,411	2.0	-1.9

On-Site Retention:

To determine the amount of run-off generated by the development of this site, analysis for the existing and developed conditions for four different storm events. These four events are for 2yr24hr, 10yr24hr, 25yr24hr, and 100yr24hr and show a maximum increase in runoff occurs in the 2yr24hr storm event with a total increase in runoff of 1,924 cubic feet. This volume will be captured by an infiltration basin as designated on the site grading plan. The total capacity of the basins must be greater than 1,924 cf. As designed, the basin will overflow and drain to the existing conditions when capacity is reached.

Mitigation:

The proposed project will include construction of a proposed commercial development including built parking, landscape, curb, gutters, ribbon gutters and design grades to direct

Specify basin detail including retention volume, dimensions, features, etc.

flows from storm water to on-site infiltration basin. Preliminary calculations determined that the on-site peak runoff for a 100-year 1-hour storm would not be increased over pre-developed conditions. It is proposed that retention be provided by above ground retention ponds as shown on the site grading plan in order to mitigate flow and reduce volume by a minimum of 1,924 cubic feet.

Conclusion:

This site can be developed as proposed with the mitigation measures stated above and meet the on-site drainage requirements of the County of San Bernardino.



Timothy A. Stark, PE 76523



This report was preliminarily approved on the following findings:

- **Curve numbers and associated soil loss rates are determined using conservative HSG.
- **Proposed retention of ΔV_{100} for mitigation.
- **Proposed retention basin dimensions seem to retain larger volume than recommended.

CALCULATION SHEET "A"

(All calculations from S.B. County Hydrology Manual)

SITE INFORMATION

DRAINAGE AREA COVERAGE

	Pre		Post	
	S.F.	%	S.F.	%
Imperv.	0	0.0%	44,152	33.6%
Landscpd	0	0.0%	27,760	21.1%
Native	131,454	100.0%	59,542	45.3%
Total	131,454		131,454	

PRECIPITATION DATA, NOAA (INCHES)

2yr-24hr	1.17
10yr-24hr	3.01
25yr-24hr	3.83
100yr-24hr	5.10
2yr-1hr	0.274
10yr-1hr	0.704
25yr-1hr	0.914
100yr-1hr	1.270

SOILS INFORMATION, WSS

Desert	Region
C	Soils Group
Barren	Native Land Cover Type

98 Assumed
for all AMC

Verify, USDA
websoils survey
indicates HSG A

CURVE NUMBERS, Cn

HSG C	AMC I	AMC II	AMC III
Imperv.	95	98	100
Landscpd	50	69	86
Native	80	91	98

Barren natural cover
seems inappropriate.
Vegetation visible on site.

Verify, Cn for residential
landscaping is 32 for
AMC II, HSG A. Modify
Cn for AMC I and III.

CALCULATION SHEET "A"

(All calculations from S.B. County Hydrology Manual)

(1000/Cn)-10

VOLUME OF RUNOFF (CF)

RUNOFF FRACTION YIELD (Y)

$$S = \frac{1000}{Cn-1}$$

$$Ia = 0.2 \times S$$

$$Y = \frac{(P24 - Ia)^2}{(P24 - Ia + S) (P24)}$$

2 YEAR - 24 HOUR STORM

Land Cover	P24 (Inches)	Cn	S	Ia	Y
Impervious	1.17	94.8	0.55	0.11	0.60
Landscaped	1.17	49.8	10.08	2.02	0.07
Native	1.17	79.8	2.53	0.51	0.12
UnDeveloped			Developed		
Y	% Coverage	(Y) By Cover	Y	% Coverage	(Y) By Cover
0.60	0%	0.00	0.60	0.34	0.20
0.07	0%	0.00	0.07	0.21	0.01
0.12	100%	0.12	0.12	0.45	0.05
UnDev Combined (Y)		0.12	Dev Combined (Y)		0.27

10 YEAR - 24 HOUR STORM

Land Cover	P24 (Inches)	Cn	S	Ia	Y
Impervious	3.01	98.0	0.20	0.04	0.92
Landscaped	3.01	69.0	4.49	0.90	0.22
Native	3.01	91.0	0.99	0.20	0.69
UnDeveloped			Developed		
Y	% Coverage	(Y) By Cover	Y	% Coverage	(Y) By Cover
0.92	0%	0.00	0.92	0.34	0.31
0.22	0%	0.00	0.22	0.21	0.05
0.69	100%	0.69	0.69	0.45	0.31
UnDev Combined (Y)		0.69	Dev Combined (Y)		0.67

See comments on page 5

CALCULATION SHEET "A"

(All calculations from S.B. County Hydrology Manual)

(1000/Cn)-10

VOLUME OF RUNOFF (CF)

RUNOFF FRACTION YIELD (Y)

$$S = \frac{1000}{Cn-1}$$

$$Ia = 0.2 \times S$$

$$Y = (P24 - Ia)^2 / (P24 - Ia + S) (P24)$$

25 YEAR - 24 HOUR STORM

Land Cover	P24 (Inches)	Cn	S	Ia	Y
Impervious	3.83	98.0	0.20	0.04	0.94
Landscaped	3.83	69.0	4.49	0.90	0.30
Native	3.83	91.0	0.99	0.20	0.75
UnDeveloped			Developed		
Y	% Coverage	(Y) By Cover	Y	% Coverage	(Y) By Cover
0.94	0%	0%	0.94	0.34	0.32
0.30	0%	0%	0.30	0.21	0.06
0.75	100%	75%	0.75	0.45	0.34
UnDev Combined (Y)		0.75	Dev Combined (Y)		0.72

100 YEAR - 24 HOUR STORM

Land Cover	P24 (Inches)	Cn	S	Ia	Y
Impervious	5.10	99.6	0.04	0.01	0.99
Landscaped	5.10	86.2	1.60	0.32	0.70
Native	5.10	98.2	0.18	0.04	0.96
UnDeveloped			Developed		
Y	% Coverage	(Y) By Cover	Y	% Coverage	(Y) By Cover
0.99	0%	0%	0.99	0.34	0.33
0.70	0%	0%	0.70	0.21	0.15
0.96	100%	96%	0.96	0.45	0.43
UnDev Combined (Y)		0.96	Dev Combined (Y)		0.91

See comments on page 5

CALCULATION SHEET "A"

(All calculations from S.B. County Hydrology Manual)

TOTAL VOLUME RUN-OFF (CF)

$$\text{Vol} = Y \times P24 \times \text{Area} / 12$$

UNDEVELOPED RUN-OFF VOLUME = $Y \times P24 \times \text{Area} / 12$

	Y	P24 (Inches)	Area (Sq. Ft.)	Run-Off (Cu. Ft.)
2 Year	0.12	1.17	131,454	1,510
10 Year	0.69	3.01	131,454	22,791
25 Year	0.75	3.83	131,454	31,273
100 Year	0.96	5.10	131,454	53,529

DEVELOPED RUN-OFF VOLUME = $Y \times P24 \times \text{Area} / 12$

	Y	P24 (Inches)	Area (Sq. Ft.)	Run-Off (Cu. Ft.)
2 Year	0.27	1.17	131,454	3,435
10 Year	0.67	3.01	131,454	22,107
25 Year	0.72	3.83	131,454	30,072
100 Year	0.91	5.10	131,454	51,117

Revise

RUN-OFF CHANGE

	2yr	10yr	25yr	100yr
Pre	1,510	22,791	31,273	53,529
Post	3,435	22,107	30,072	51,117
Increase	1,924	-684	-1,201	-2,411
Largest Increase				1,924

CALCULATION SHEET "A"

(All calculations from S.B. County Hydrology Manual)

100 YEAR STORM - PEAK RUNOFF FLOW, Q (CFS)

LOSS RATE, F_m (IN/HR)

$F_m = A_p \times F_p$ (See Plate C-6 Attached for F_p)

These are area averaged values. Perviousness was not factored in.

F_m Pre Development Area (Native)

	F_p (InfilCap)	Pre A_p (S.F. %)	Pre F_m (In/hr)
Imperv.	0.00	0.0%	0.000
Landscpd	0.59	0.0%	0.000
Native	0.18	100.0%	0.180
Total Site PreDev F_m			0.180

See comments on page 5

Post Development Area (Impervious/Landscaped/Native)

	F_p (InfilCap)	Post A_p (S.F. %)	Post F_m (In/hr)
Imperv.	0.00	33.6%	0.000
Landscpd	0.59	21.1%	0.125
Native	0.18	45.3%	0.082
Total Site PostDev F_m			0.206

Revised F_p is ok, existing assumed to be 100% pervious.
 $F_m = F_p \times 1 = F_p$

Revise

RAINFALL INTENSITY, I (IN/HR)

	I , 100yr	
PreDev	4.1	(See Plate D-3 Attached)
PostDev	3.4	(See Plate D-3 Attached)

This is not pervious factor a_p . This is the composition of land cover as percentage of total site area.

TIME OF CONCENTRATION, T_c (MINUTES)

PreDev (min)	13.1	(See Plate D-1 Attached)
PostDev $T_c = T_c$ Area 1 + T_c Area 2		
PostDev $T_c = 6.8 + 8.3$		
PostDev (min)	15.1	(See Plates D-1 Attached)

See comment in intensity-duration graph.

CALCULATION SHEET "A"

(All calculations from S.B. County Hydrology Manual)

100 YEAR STORM - PEAK RUNOFF FLOW, Q (CFS)

Pre Developed Q (CFS)

$$Q = \text{Area} \times 0.9 \times (I - F_m)$$

$$Q_{\text{PreDev}} = 131,454 / 43,560 \times 0.9 \times (4.1 - 0.180)$$

$$Q_{\text{PreDev}} = 10.6 \text{ CFS}$$

Post Developed Q (CFS)

$$Q = \text{Area} \times 0.9 \times (I - F_m)$$

$$Q_{\text{PostDev1}} = (131,454) / 43,560 \times 0.9 \times (3.4 - 0.206)$$

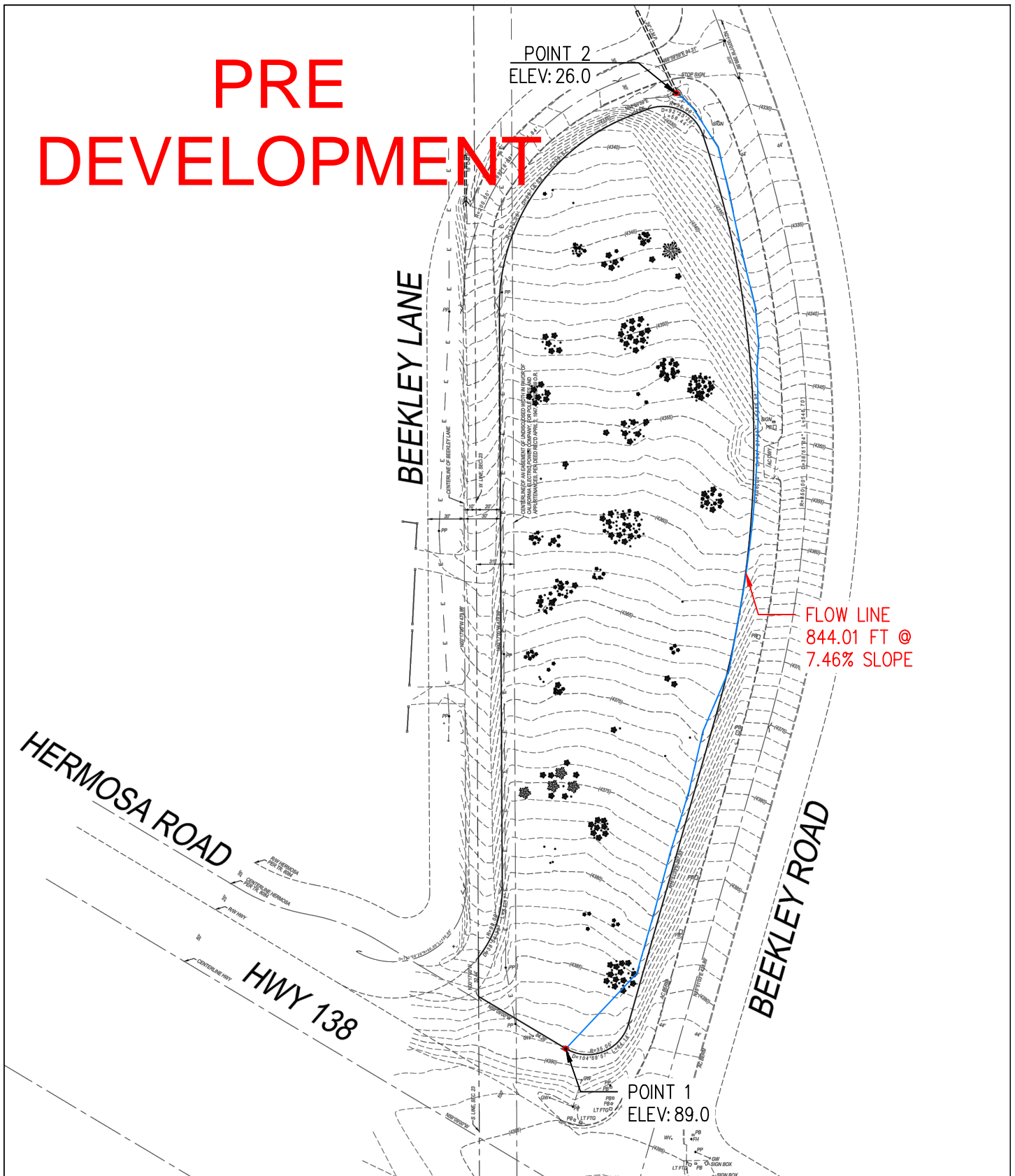
$$Q_{\text{PostDev1}} = 8.7 \text{ CFS}$$

Post development plan has two separate drainage areas. Follow Hydrology Manual Section D.11 for methodology on determining total site peak flow rate.

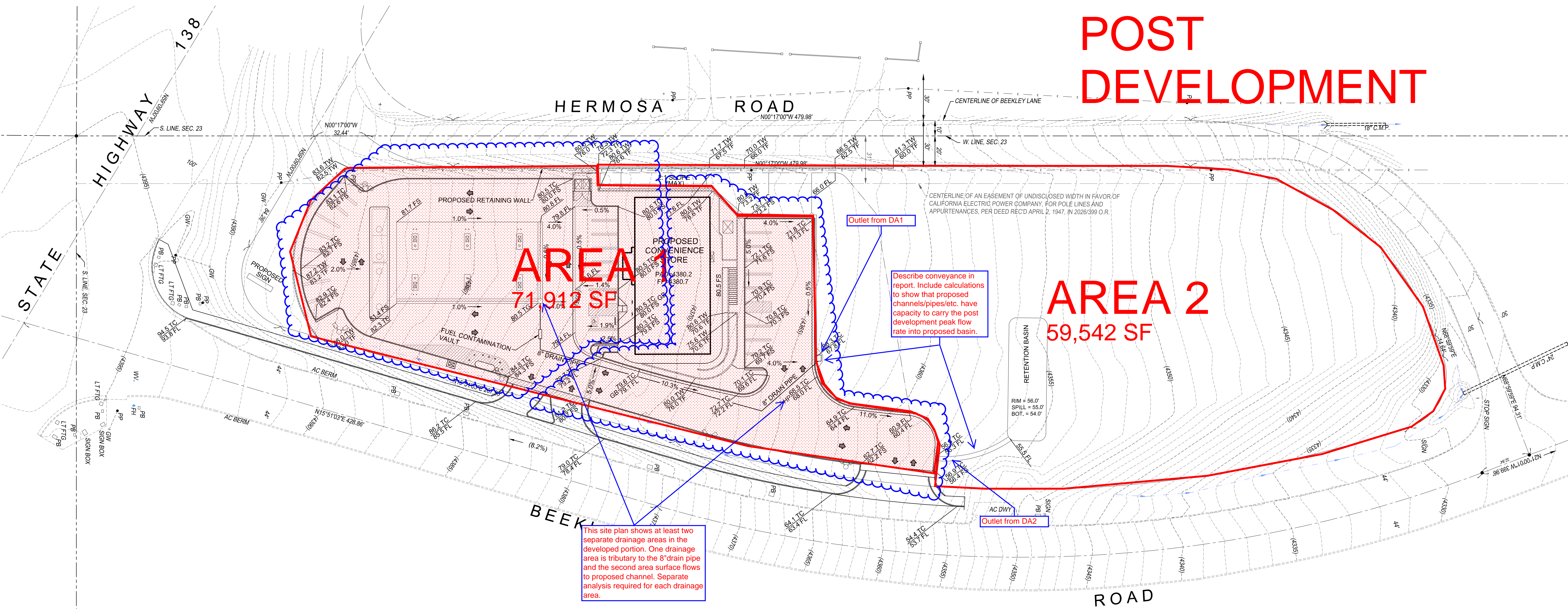
Change in Q = -1.9 CFS

The above referenced equations should **increase** the post development peak flow rate. However, the proposed site improvements will capture the full difference in storm runoff volume from pre and post development conditions. The report should make statement that no increase in peak flow rate is expected because the difference in runoff volume generated in the post development condition will be retained on site.

PRE DEVELOPMENT



POST
DEVELOPMENT



OWNER:
GEORGE WANIS
9128 GREEN ROAD
PINON HILLS, CA 92372
(661) 860-0226

ARCHITECT:
STEENO DESIGN STUDIO, INC.
11774 HESPERIA ROAD #B2
HESPERIA, CA 92345
(760) 244-5001

ENGINEER:
HIGH DESERT MAPPING
16704 NEENACH ROAD
APPLE VALLEY, CA 92307
(760) 508-8555

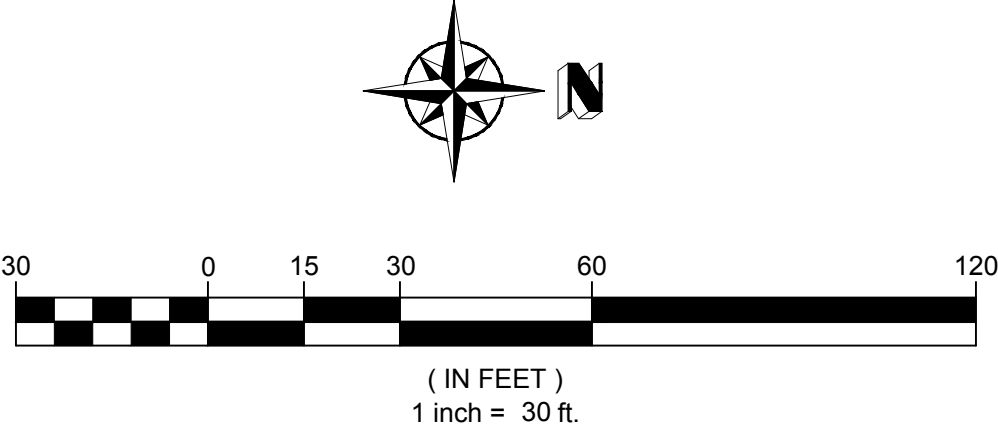
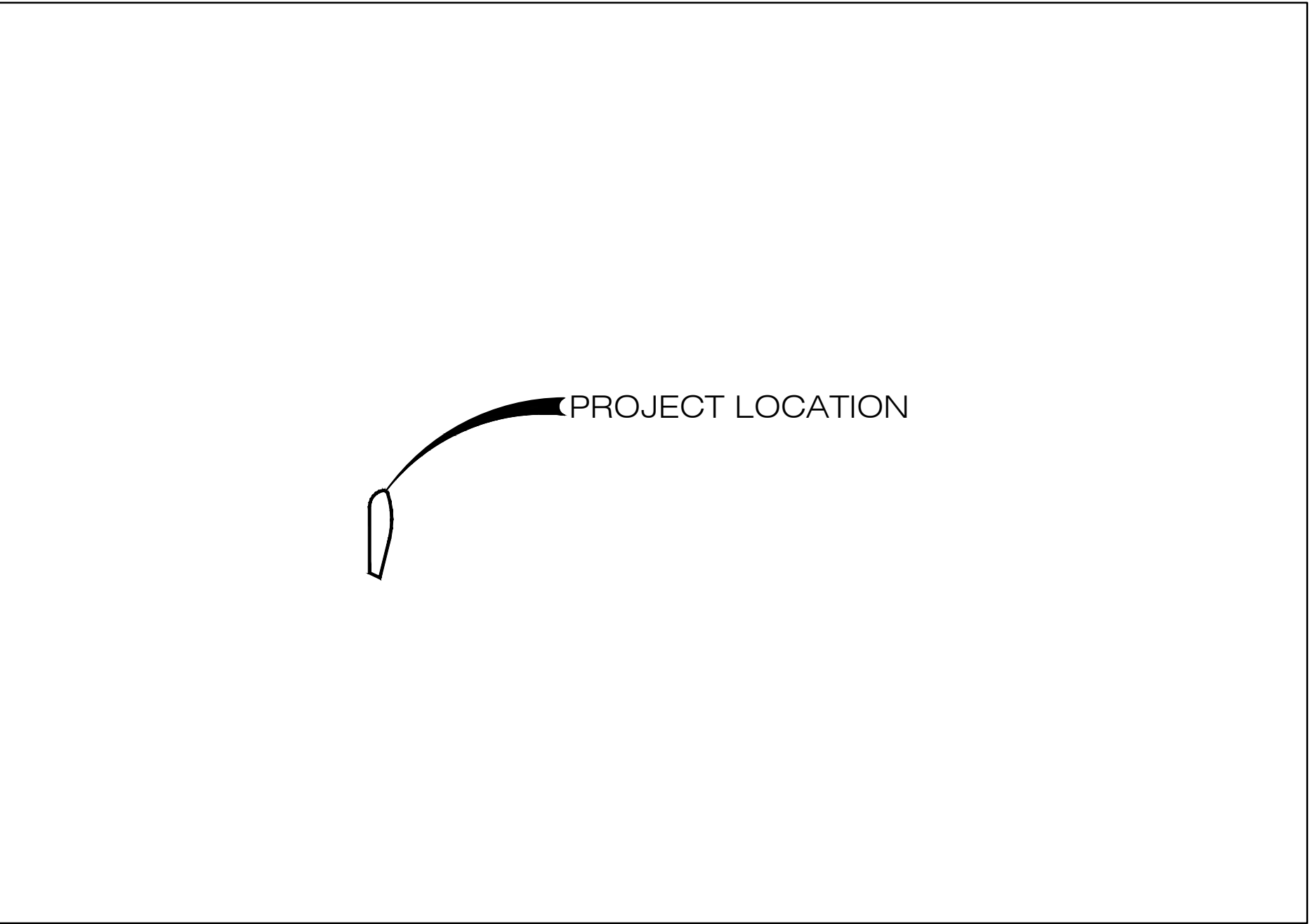


STATISTICS:
APN 3066-191-04
3.02 AC (NET)

LEGAL DESCRIPTION:
BEING A PORTION OF THE SOUTHWEST
ONE-QUARTER OF SECTION 23, T4N, R7W, SBM.

BENCHMARK:
COUNTY OF SAN BERNARDINO BM
A 2" BRASS CAP FOR THE W 1/4 CORNER OF SECTION 23,
LOCATED AT THE INTERSECTION OF BEEKLEY ROAD
AND NIELSON ROAD, PER CSFB 4150/850

ELEVATION: 4250.85



IN THE COUNTY OF SAN BERNARDINO
CONCEPTUAL GRADING PLAN
PROPOSED GAS STATION
& CONVENIENCE STORE
BEEKLEY ROAD, PHELAN

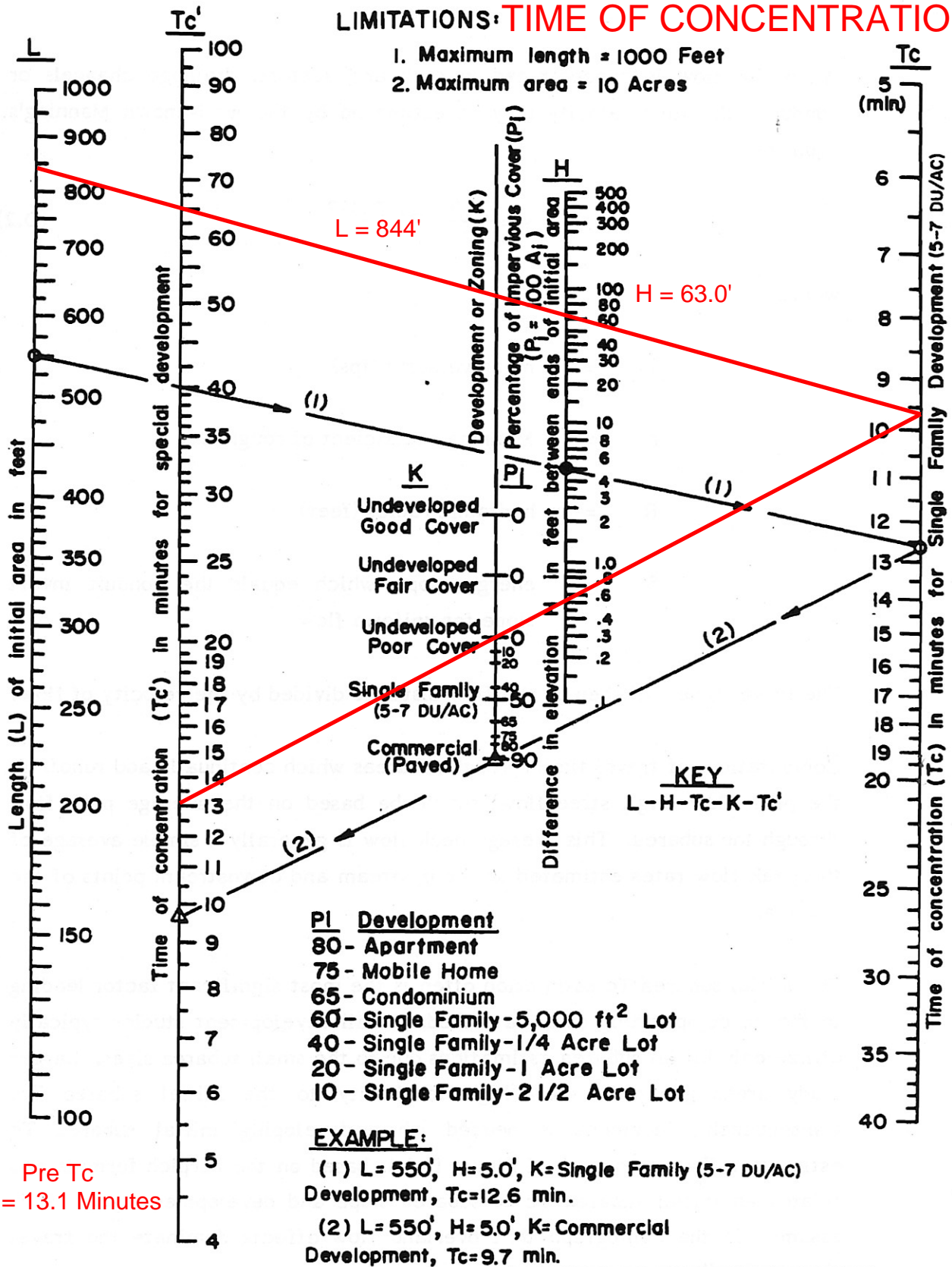


[illegible]

Know what's below.
Call before you dig.

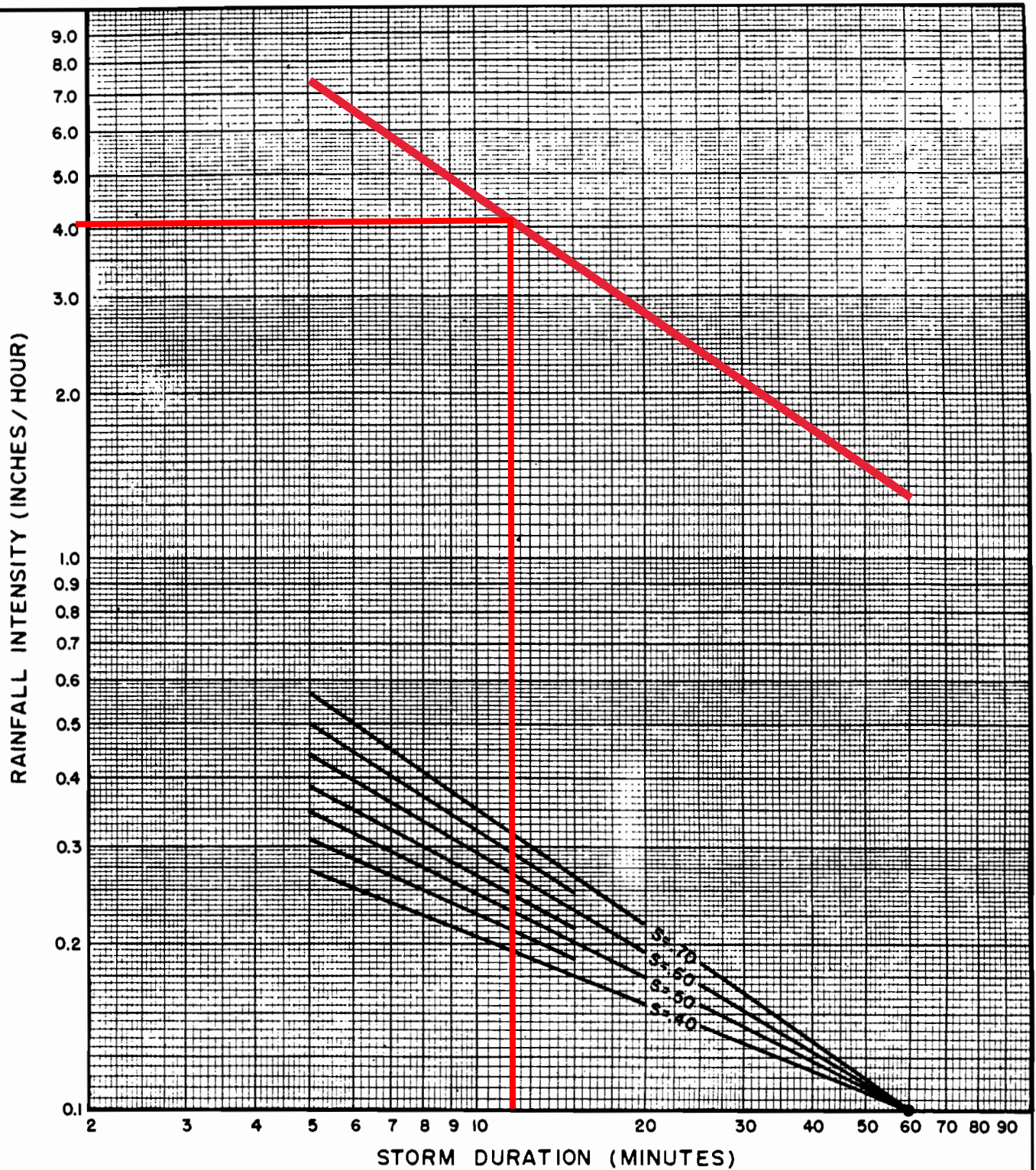
LIMITATIONS: TIME OF CONCENTRATION

1. Maximum length = 1000 Feet
2. Maximum area = 10 Acres



SAN BERNARDINO COUNTY
HYDROLOGY MANUAL

TIME OF CONCENTRATION
NOMOGRAPH
FOR INITIAL SUBAREA



PRE-DEVELOPMENT

Design Storm Frequency = 100 year
 One Hour Point Rainfall = 1.27 inches
 LOG-LOG Slope = 0.7
 $T_c = 13.1$ mins
 Project Location = Apple Valley CA.

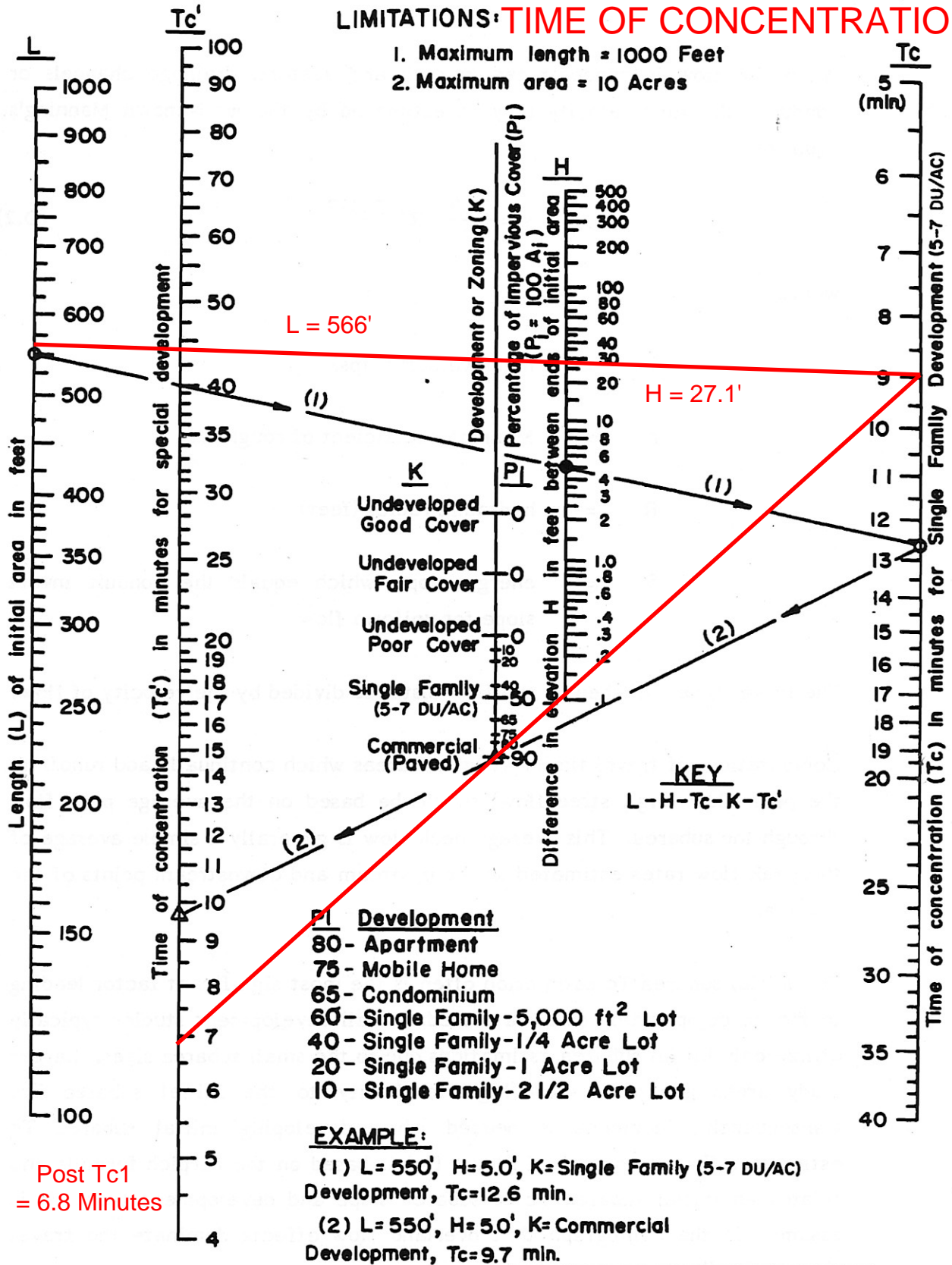
$I = 4.1$ in/hr

SAN BERNARDINO COUNTY
 HYDROLOGY MANUAL

INTENSITY - DURATION
 CURVES
 CALCULATION SHEET

LIMITATIONS: TIME OF CONCENTRATION

1. Maximum length = 1000 Feet
2. Maximum area = 10 Acres

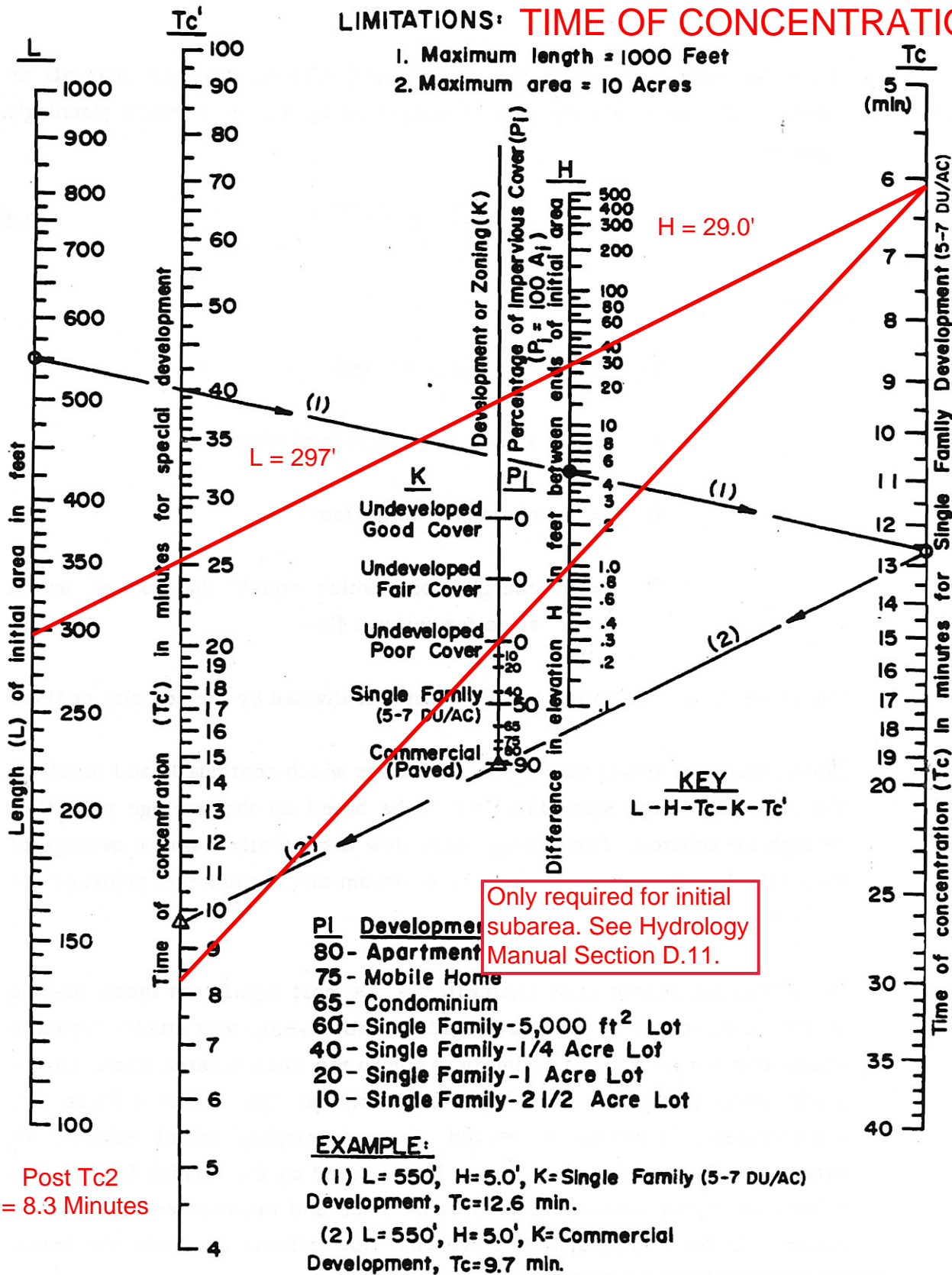


SAN BERNARDINO COUNTY
HYDROLOGY MANUAL

TIME OF CONCENTRATION
NOMOGRAPH
FOR INITIAL SUBAREA

LIMITATIONS: TIME OF CONCENTRATION

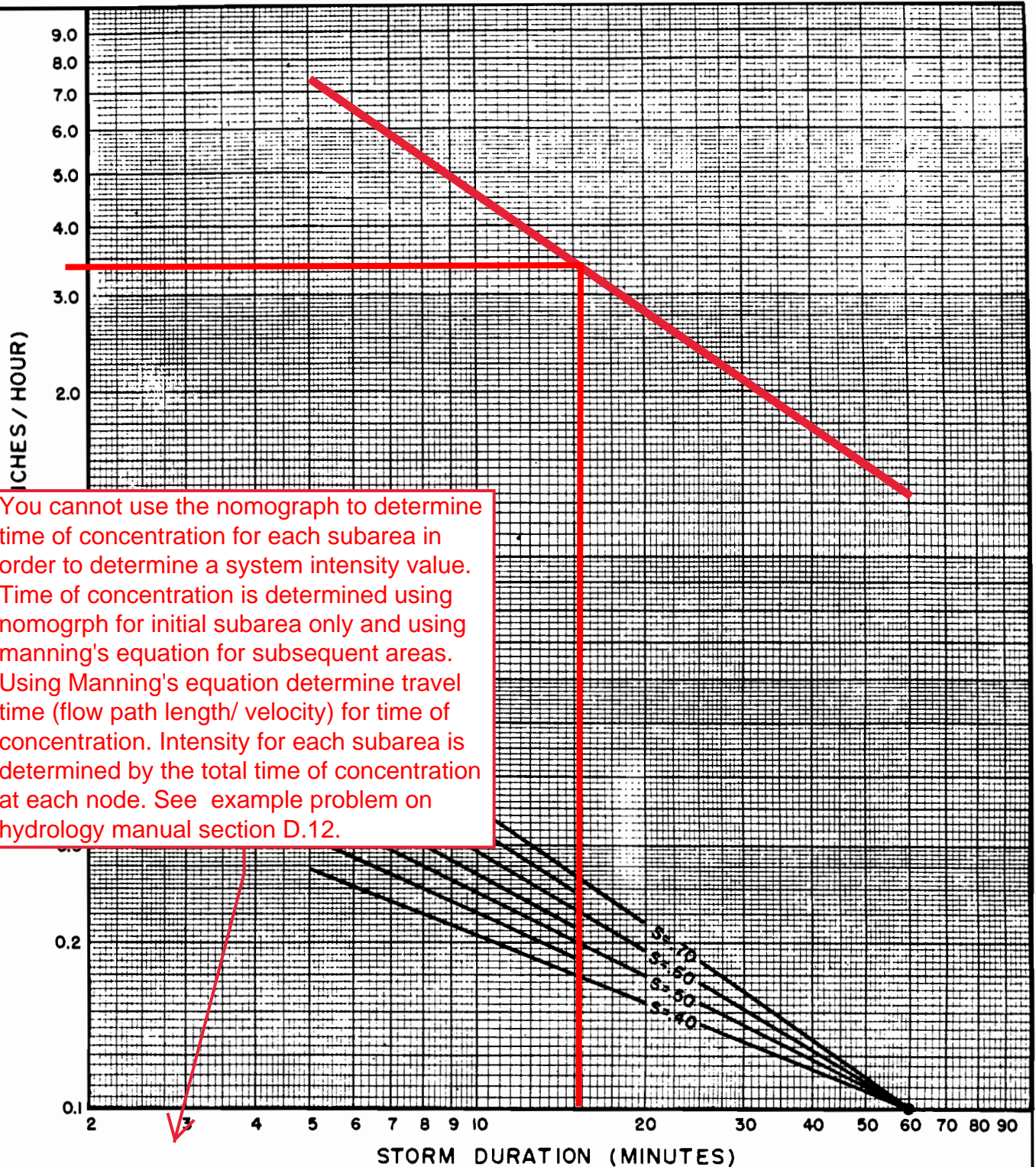
1. Maximum length = 1000 Feet
2. Maximum area = 10 Acres



SAN
HYDR

This nomograph is for initial subarea only. Follow section D.11 of hydrology manual for methodology to determine peak flow rate at different points of concentration.

TIME OF CONCENTRATION
NOMOGRAPH
FOR INITIAL SUBAREA



SAN BERNARDINO COUNTY
 HYDROLOGY MANUAL

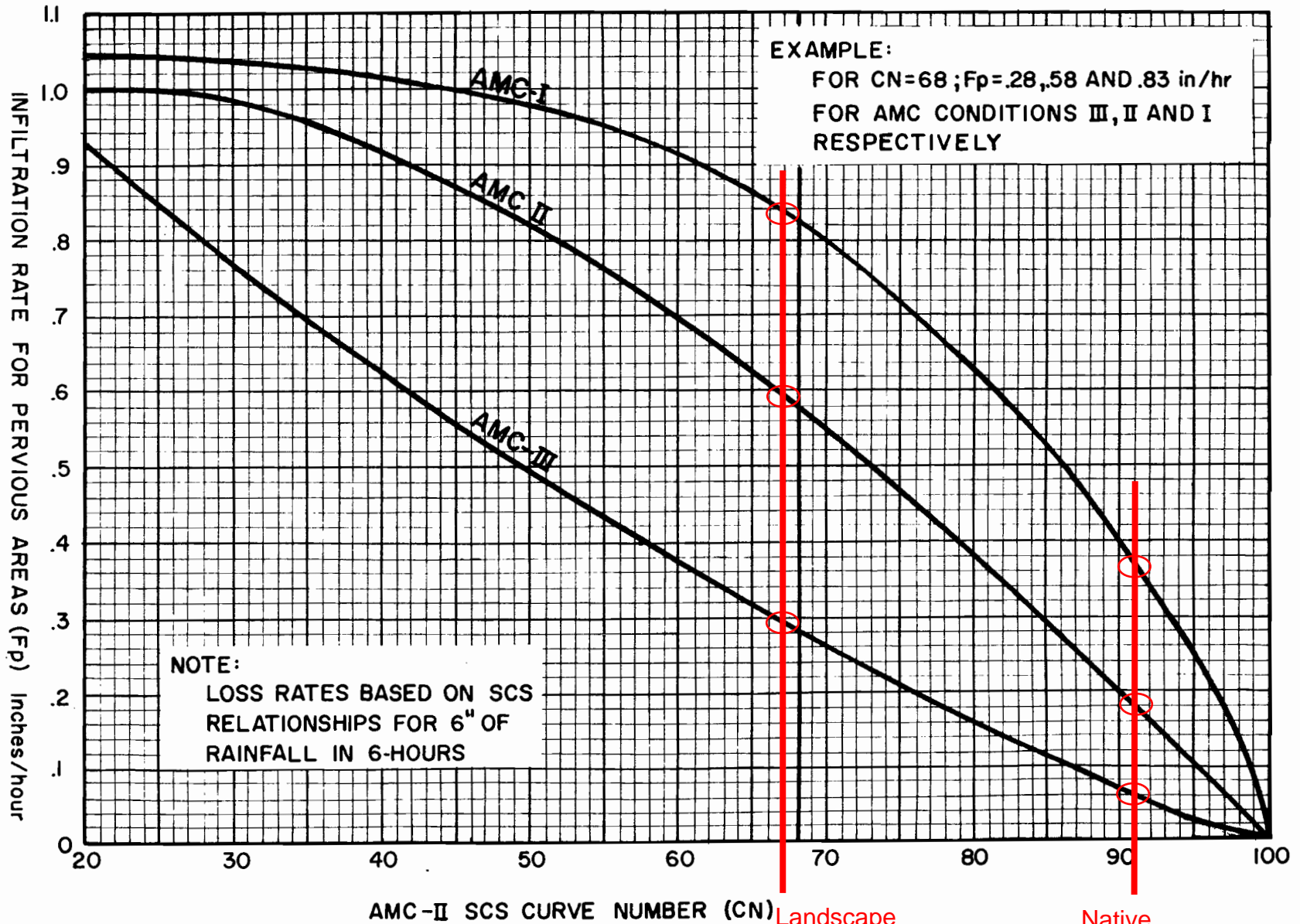
**INTENSITY - DURATION
 CURVES
 CALCULATION SHEET**

Revise for HSG A

Landscape
 $F_p=0.84$, I
 $F_p=0.59$, II
 $F_p=0.29$, III

Native
 $F_p=0.36$, I
 $F_p=0.18$, II
 $F_p=0.06$, III

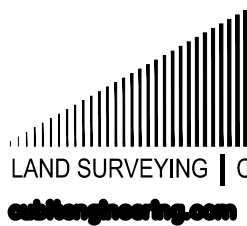
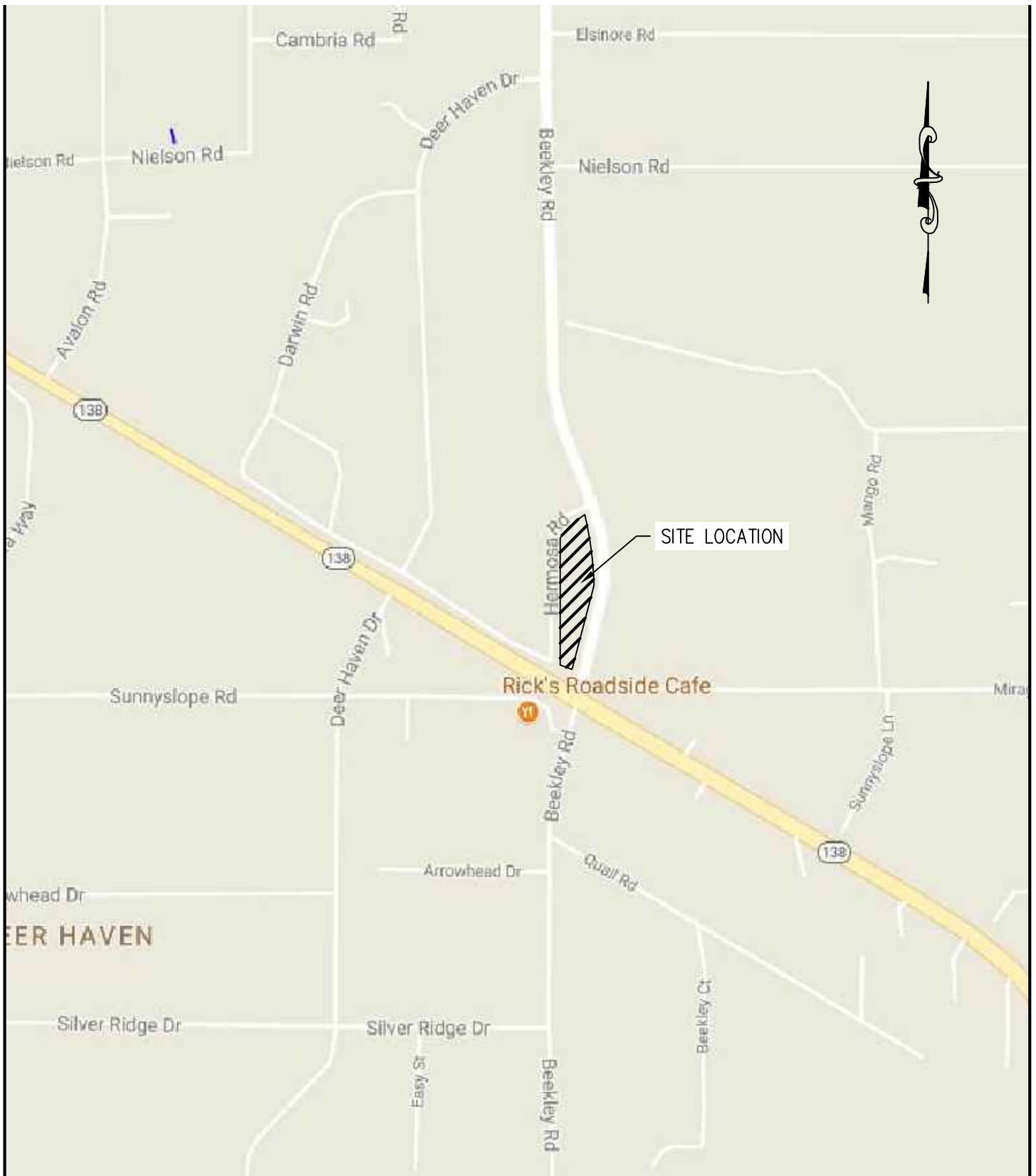
SAN BERNARDINO COUNTY HYDROLOGY MANUAL



Landscape
 $C_n = 69$, AMC II

Native
 $C_n = 91$, AMC II

INTEGRATION RATE FOR
PERVIOUS AREAS VERSUS
SCS CURVE NUMBERS



CUBIT ENGINEERING

16490 WALNUT STREET, SUITE B-3
HESPERIA, CA 92345 (760) 244-2247

LAND SURVEYING | CIVIL ENGINEERING | CONSTRUCTION

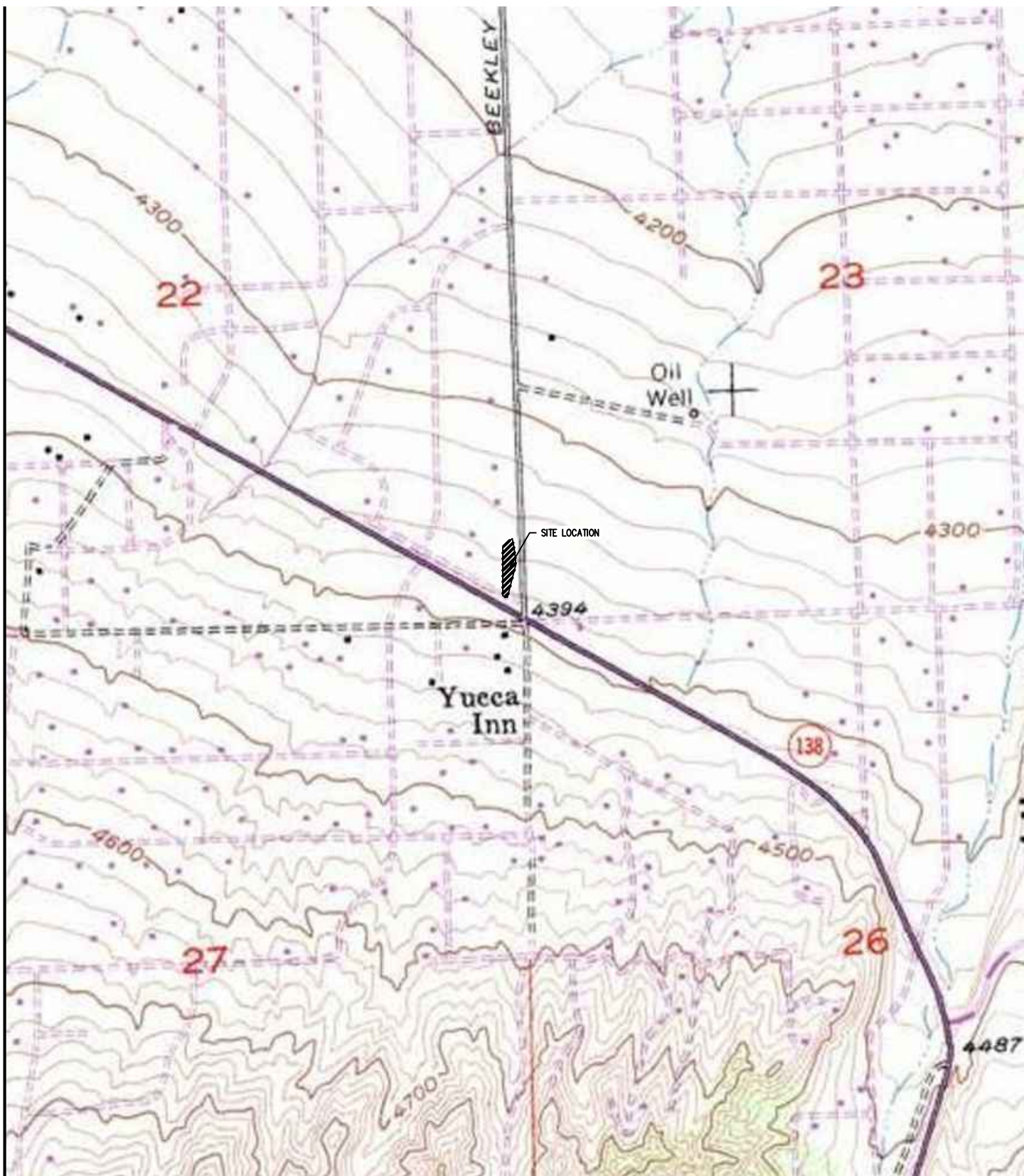
cubitengineering.com

**VICINITY MAP
NOT TO SCALE**

APN 3066-191-04

2016.0225

MAP - 1



CUBIT ENGINEERING

16490 WALNUT STREET, SUITE B-3
HESPERIA, CA 92345 (760) 244-2247

LAND SURVEYING | CIVIL ENGINEERING | CONSTRUCTION

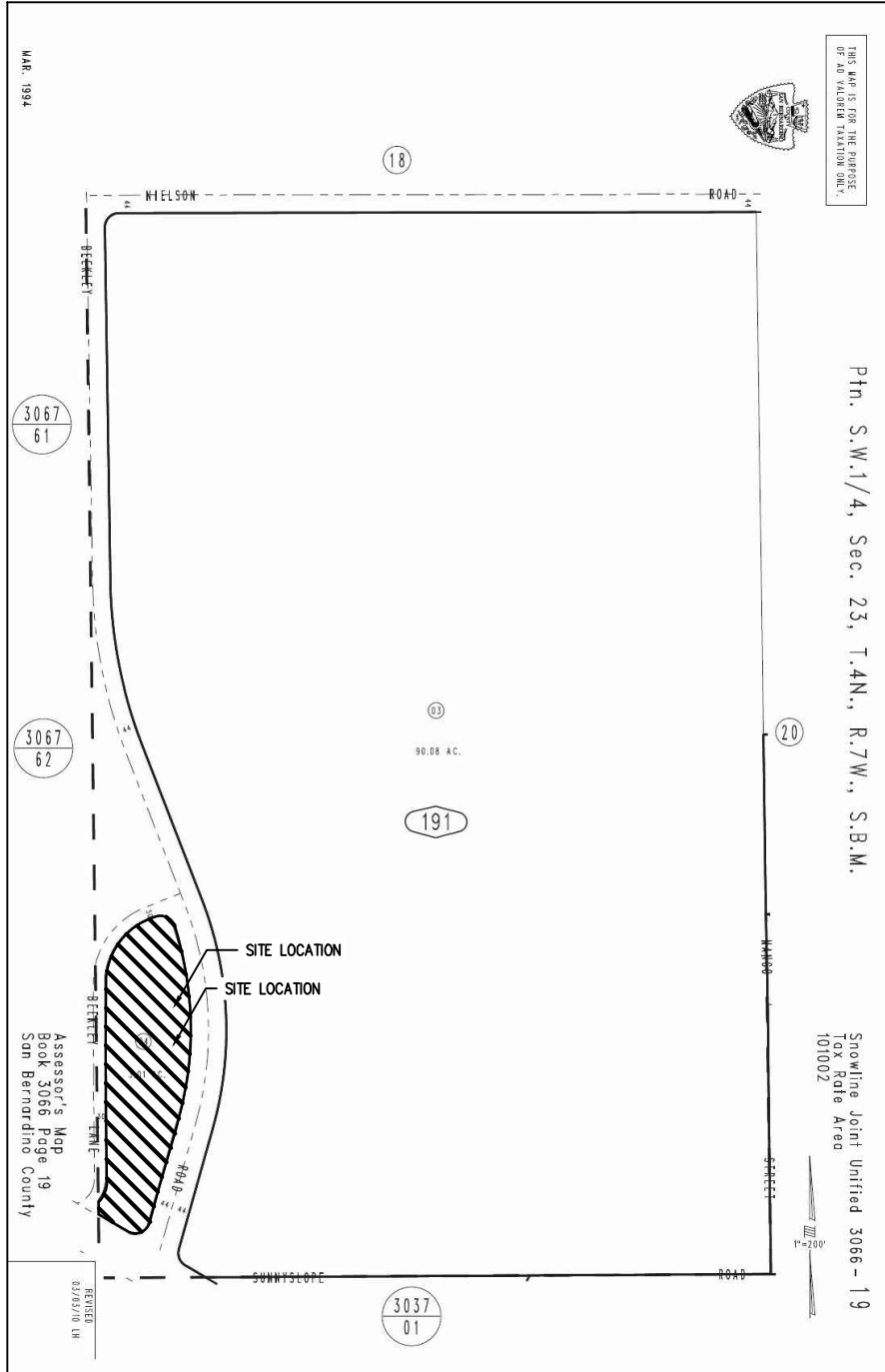
cubitengineering.com

TOPOGRAPHIC MAP
NOT TO SCALE

APN 3066-191-04

2016.0225

MAP - 2

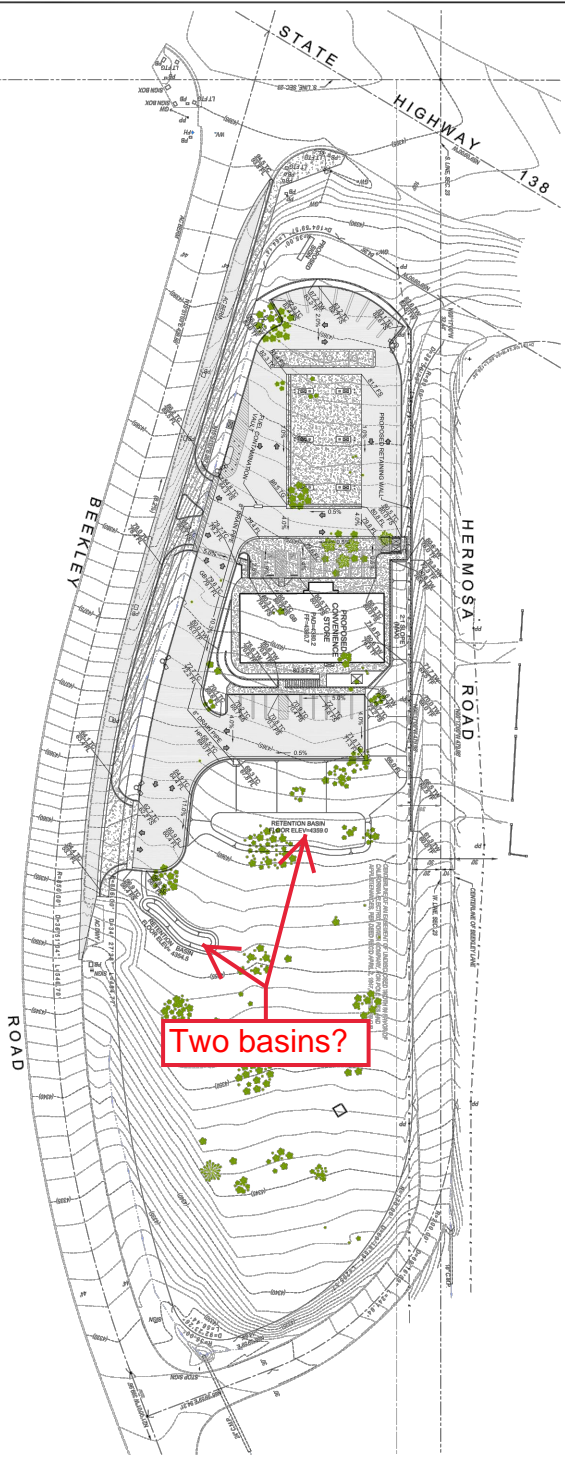


ASSESSOR'S MAP NOT TO SCALE

APN 3066-191-04

2016.0225

MAP - 3



OWNER
 THE HESPERIA LAND TRUST
 16490 WALNUT STREET, SUITE B-3
 HESPERIA, CA 92345
 (760) 244-2247

ARCHITECT
 STEVEN GORDON ARCHITECT, INC.
 16490 WALNUT STREET, SUITE B-3
 HESPERIA, CA 92345
 (760) 244-2247

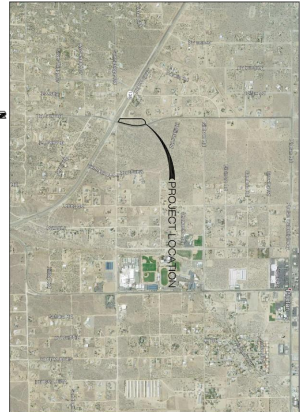
ENGINEER
 CUBIT ENGINEERING
 16490 WALNUT STREET, SUITE B-3
 HESPERIA, CA 92345
 (760) 244-2247

UTILITY SUPPLIERS
 WATER: HESPERIA PUBLIC UTILITY
 16490 WALNUT STREET, SUITE B-3
 HESPERIA, CA 92345
 (760) 244-2247
 SEWER: HESPERIA PUBLIC UTILITY
 16490 WALNUT STREET, SUITE B-3
 HESPERIA, CA 92345
 (760) 244-2247
 ELECTRIC: SOUTHERN CALIFORNIA Edison
 16490 WALNUT STREET, SUITE B-3
 HESPERIA, CA 92345
 (760) 244-2247
 TELEPHONE: SOUTHERN CALIFORNIA Edison
 16490 WALNUT STREET, SUITE B-3
 HESPERIA, CA 92345
 (760) 244-2247
 GAS: SOUTHERN CALIFORNIA Edison
 16490 WALNUT STREET, SUITE B-3
 HESPERIA, CA 92345
 (760) 244-2247

STATISTICS
 3.00 AC TOTAL
 3.00 AC TOTAL
 3.00 AC TOTAL

LEGAL DESCRIPTION:
 BEING A PORTION OF THE ACQUANT
 ONE-QUARTER OF SECTION 22, T4N, 37E, R2E, SD45
 COUNTY OF SAN BERNARDINO, CALIF.

BENCHMARK:
 A TYPICAL CAP PILE 10" IN DIAMETER OF SECTION 22,
 T4N, 37E, R2E, SD45, BEING A PORTION OF THE ACQUANT
 ONE-QUARTER OF SECTION 22, T4N, 37E, R2E, SD45
 COUNTY OF SAN BERNARDINO, CALIF.



NOT TO SCALE



IN THE COUNTY OF SAN BERNARDINO
 CONCEPTUAL GRADING PLAN
 PROPOSED GAS STATION
 & CONVENIENCE STORE
 BEEKLEY ROAD, PHELAN



Cubit Engineering

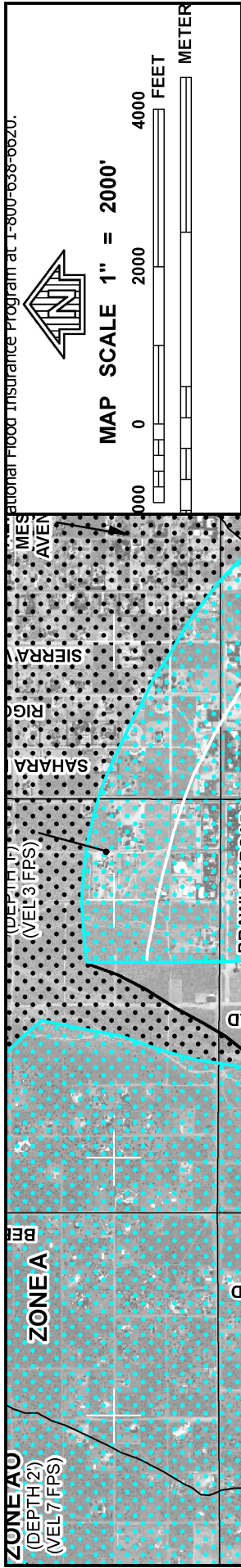
CONCEPT GRADING PLAN
 NOT TO SCALE

APN 3066-191-04

2016.0225

MAP - 4

**CUBIT
ENGINEERING**
 16490 WALNUT STREET, SUITE B-3
 HESPERIA, CA 92345 (760) 244-2247
 LAND SURVEYING | CIVIL ENGINEERING | CONSTRUCTION
 cubitengineering.com



NFIP NATIONAL FLOOD INSURANCE PROGRAM

PANEL 6450H

FIRM
FLOOD INSURANCE RATE MAP
SAN BERNARDINO
COUNTY,
CALIFORNIA
AND INCORPORATED AREAS
PANEL 6450 OF 9400
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:
COMMUNITY
SAN BERNARDINO COUNTY
NUMBER 060270
PANEL 6450
SUFFIX H

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.



MAP NUMBER
06071C6450H
MAP REVISED
AUGUST 28, 2008

Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov



NOAA Atlas 14, Volume 6, Version 2
Location name: Phelan, California, USA*
Latitude: 34.413°, Longitude: -117.5896°
Elevation: 4365.27 ft**

* source: ESRI Maps

** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

[PF_tabular](#) | [PF_graphical](#) | [Maps_&_aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.081 (0.067-0.098)	0.115 (0.095-0.141)	0.165 (0.136-0.201)	0.207 (0.169-0.255)	0.268 (0.212-0.343)	0.319 (0.247-0.416)	0.373 (0.282-0.498)	0.431 (0.317-0.592)	0.514 (0.362-0.737)	0.581 (0.396-0.863)
10-min	0.115 (0.096-0.141)	0.165 (0.137-0.202)	0.236 (0.195-0.289)	0.296 (0.243-0.366)	0.385 (0.305-0.491)	0.457 (0.354-0.596)	0.534 (0.404-0.714)	0.618 (0.454-0.849)	0.737 (0.519-1.06)	0.833 (0.567-1.24)
15-min	0.140 (0.116-0.170)	0.200 (0.166-0.244)	0.285 (0.235-0.349)	0.359 (0.293-0.442)	0.465 (0.368-0.594)	0.553 (0.428-0.721)	0.646 (0.488-0.864)	0.747 (0.549-1.03)	0.891 (0.628-1.28)	1.01 (0.686-1.50)
30-min	0.200 (0.166-0.244)	0.286 (0.237-0.350)	0.408 (0.337-0.500)	0.513 (0.420-0.634)	0.666 (0.527-0.851)	0.791 (0.613-1.03)	0.925 (0.699-1.24)	1.07 (0.786-1.47)	1.28 (0.899-1.83)	1.44 (0.982-2.14)
60-min	0.274 (0.227-0.334)	0.393 (0.325-0.480)	0.560 (0.462-0.686)	0.704 (0.576-0.869)	0.914 (0.723-1.17)	1.09 (0.841-1.42)	1.27 (0.960-1.70)	1.47 (1.08-2.02)	1.75 (1.23-2.51)	1.98 (1.35-2.94)
2-hr	0.405 (0.335-0.493)	0.556 (0.460-0.678)	0.765 (0.632-0.937)	0.945 (0.773-1.17)	1.20 (0.952-1.54)	1.41 (1.09-1.84)	1.63 (1.23-2.18)	1.87 (1.37-2.57)	2.20 (1.55-3.15)	2.46 (1.68-3.66)
3-hr	0.500 (0.414-0.609)	0.676 (0.560-0.825)	0.919 (0.759-1.13)	1.13 (0.922-1.39)	1.42 (1.13-1.81)	1.66 (1.29-2.16)	1.91 (1.44-2.55)	2.18 (1.60-2.99)	2.55 (1.80-3.66)	2.84 (1.93-4.22)
6-hr	0.709 (0.588-0.864)	0.951 (0.787-1.16)	1.28 (1.06-1.57)	1.56 (1.28-1.92)	1.95 (1.54-2.49)	2.26 (1.75-2.95)	2.59 (1.96-3.46)	2.93 (2.15-4.03)	3.41 (2.40-4.89)	3.79 (2.58-5.62)
12-hr	0.932 (0.773-1.14)	1.30 (1.08-1.59)	1.79 (1.48-2.19)	2.20 (1.80-2.71)	2.76 (2.10-3.53)	3.21 (2.48-4.18)	3.66 (2.77-4.89)	4.14 (3.04-5.69)	4.79 (3.37-6.87)	5.30 (3.60-7.86)
24-hr	1.17 (1.03-1.34)	1.71 (1.51-1.97)	2.42 (2.14-2.80)	3.01 (2.64-3.51)	3.83 (3.24-4.61)	4.46 (3.70-5.48)	5.10 (4.13-6.43)	5.77 (4.55-7.48)	6.69 (5.06-9.03)	7.40 (5.41-10.3)
2-day	1.34 (1.19-1.54)	1.99 (1.76-2.29)	2.85 (2.52-3.29)	3.56 (3.12-4.15)	4.55 (3.85-5.47)	5.31 (4.41-6.53)	6.10 (4.94-7.68)	6.92 (5.45-8.96)	8.03 (6.07-10.8)	8.90 (6.50-12.4)
3-day	1.45 (1.28-1.67)	2.15 (1.90-2.48)	3.11 (2.75-3.59)	3.90 (3.42-4.55)	5.00 (4.23-6.02)	5.85 (4.86-7.20)	6.73 (5.45-8.48)	7.66 (6.03-9.92)	8.92 (6.74-12.0)	9.92 (7.25-13.9)
4-day	1.53 (1.35-1.76)	2.28 (2.02-2.63)	3.32 (2.93-3.84)	4.18 (3.66-4.87)	5.38 (4.56-6.48)	6.32 (5.24-7.77)	7.30 (5.91-9.19)	8.32 (6.55-10.8)	9.74 (7.36-13.1)	10.9 (7.93-15.2)
7-day	1.67 (1.48-1.92)	2.51 (2.22-2.90)	3.70 (3.27-4.28)	4.72 (4.13-5.50)	6.16 (5.22-7.42)	7.31 (6.06-8.99)	8.51 (6.89-10.7)	9.79 (7.71-12.7)	11.6 (8.76-15.7)	13.0 (9.51-18.2)
10-day	1.68 (1.49-1.94)	2.54 (2.25-2.93)	3.78 (3.34-4.37)	4.85 (4.25-5.65)	6.40 (5.43-7.71)	7.65 (6.35-9.41)	8.97 (7.27-11.3)	10.4 (8.18-13.5)	12.4 (9.38-16.7)	14.0 (10.2-19.6)
20-day	1.89 (1.67-2.17)	2.93 (2.59-3.38)	4.47 (3.95-5.17)	5.84 (5.11-6.80)	7.87 (6.67-9.48)	9.55 (7.93-11.7)	11.4 (9.21-14.3)	13.3 (10.5-17.3)	16.2 (12.2-21.8)	18.5 (13.5-25.8)
30-day	2.28 (2.02-2.63)	3.53 (3.12-4.07)	5.40 (4.77-6.24)	7.08 (6.20-8.25)	9.62 (8.15-11.6)	11.7 (9.75-14.4)	14.0 (11.4-17.7)	16.6 (13.0-21.5)	20.2 (15.3-27.3)	23.2 (17.0-32.4)
45-day	2.56 (2.27-2.95)	3.96 (3.51-4.57)	6.09 (5.37-7.04)	8.03 (7.03-9.35)	11.0 (9.32-13.3)	13.5 (11.2-16.6)	16.3 (13.2-20.5)	19.3 (15.2-24.9)	23.6 (17.8-31.9)	27.2 (19.9-38.0)
60-day	2.81 (2.49-3.23)	4.29 (3.80-4.95)	6.59 (5.82-7.62)	8.70 (7.62-10.1)	11.9 (10.1-14.4)	14.7 (12.2-18.1)	17.8 (14.4-22.4)	21.1 (16.6-27.3)	25.9 (19.6-34.9)	29.8 (21.8-41.6)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

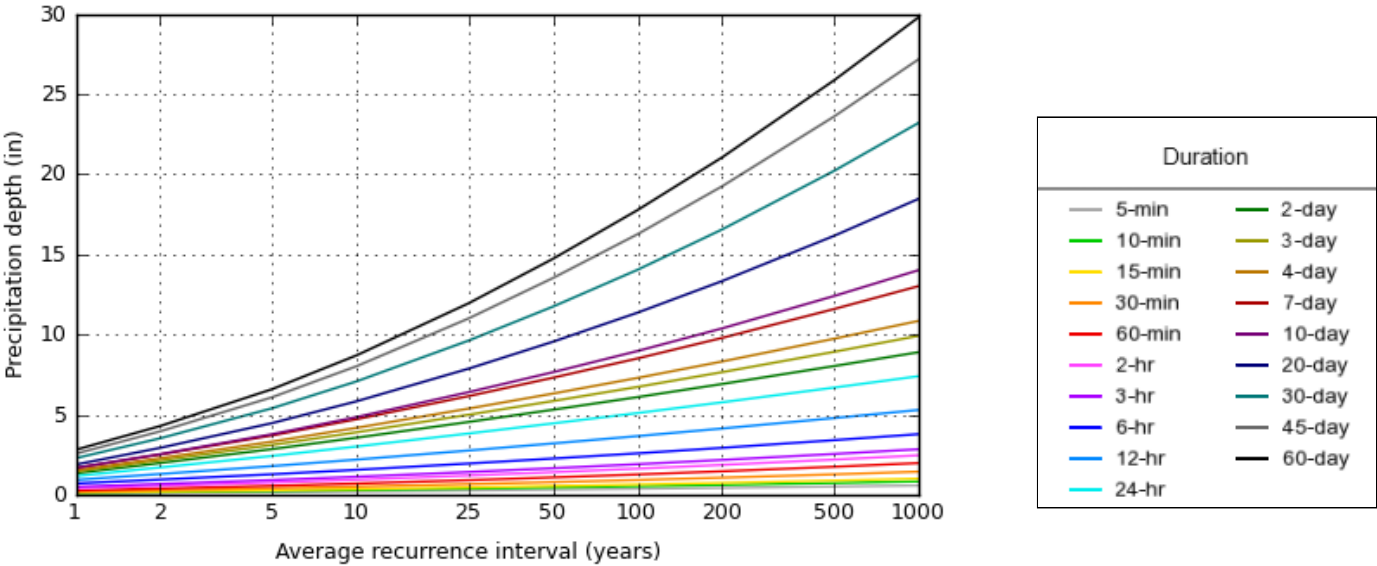
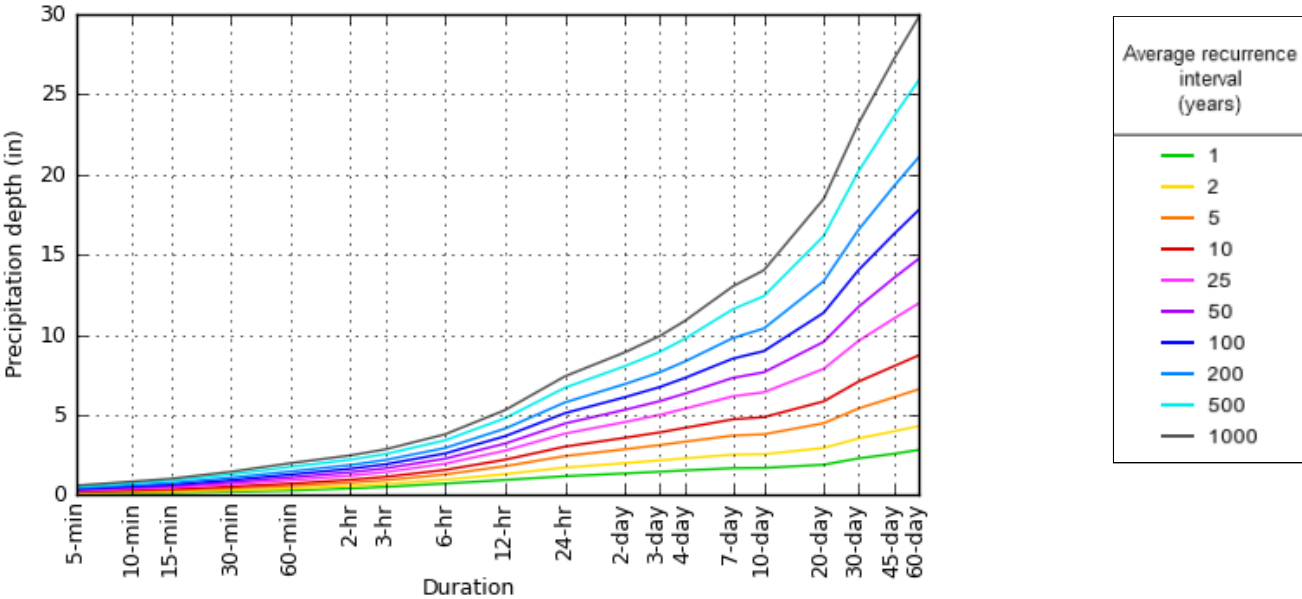
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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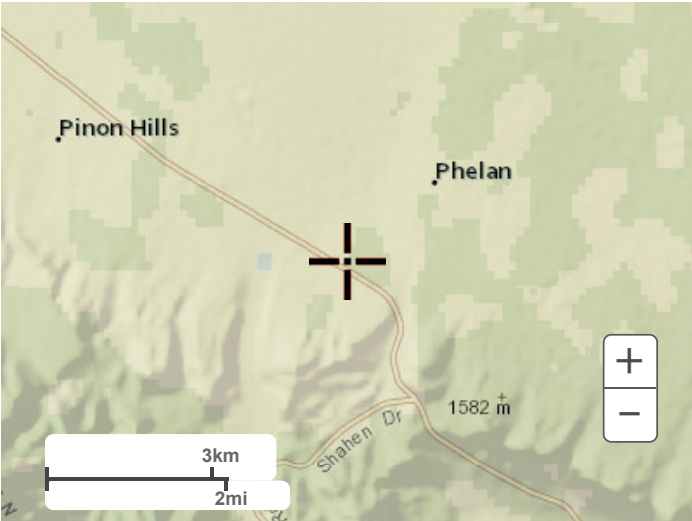
PF graphical

PDS-based depth-duration-frequency (DDF) curves
Latitude: 34.4130°, Longitude: -117.5896°



Maps & aerials

Small scale terrain



Large scale terrain



Large scale map



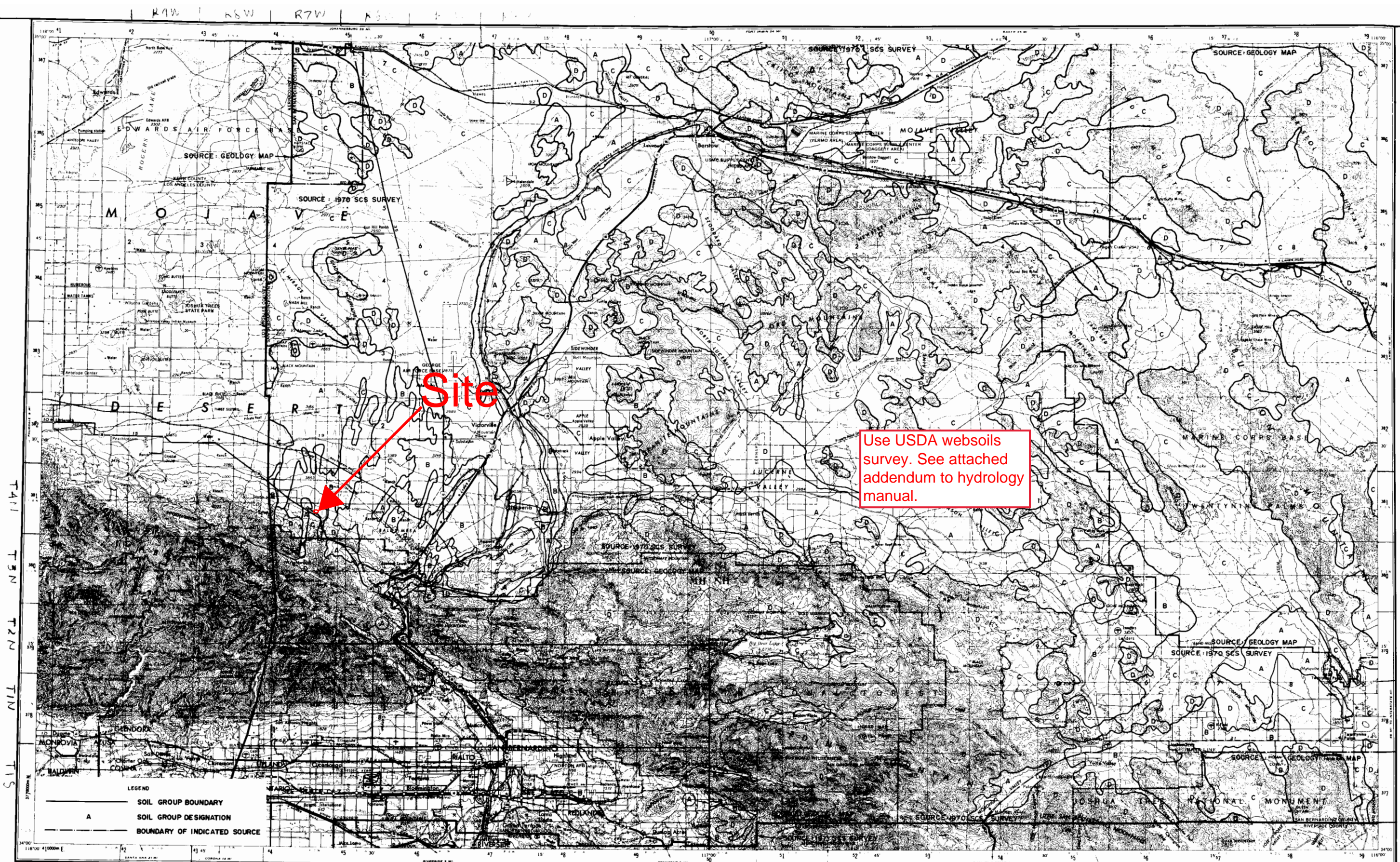
Large scale aerial



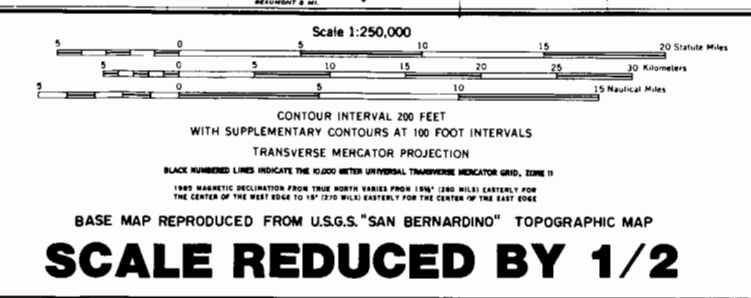
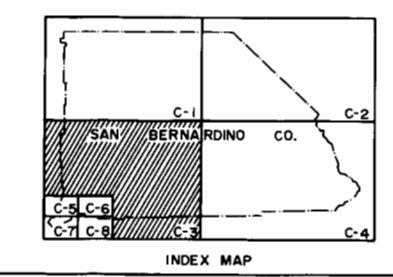
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1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

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SAN BERNARDINO COUNTY **HYDROLOGY MANUAL**



HYDROLOGIC SOILS GROUP MAP **FOR** **SOUTHCENTRAL AREA**

DEPARTMENT OF PUBLIC WORKS

FLOOD CONTROL • LAND DEVELOPMENT & CONSTRUCTION
SOLID WASTE MANAGEMENT • SURVEYOR • TRANSPORTATION

COUNTY OF SAN BERNARDINO

825 East Third Street • San Bernardino, CA 92415-0835 • (909) 387-8104
Fax (909) 387-8130

GRANVILLE M. "BOW" BOWMAN, P.E., P.L.S.
Director of Public Works

April 6, 2010

File: 1(FC)-27.09-01

Hydrology Manual User:

Our San Bernardino County Hydrology Manual was developed in 1983 and revised in 1986 by our consultant Dr. Ted Hromadka. The best available data at the time was the National Oceanic and Atmospheric Administration (NOAA) Atlas II rainfall records and statistics published in 1973; it was the basis of our manual. The County participated with NOAA by providing a portion of the funding to study an additional 30 years of rainfall records. NOAA Atlas XIV was published in 2004 and revised 2006. We asked Dr. Hromadka to review the new rainfall numbers and assess any impacts to our manual. We looked at all areas of the County to see if any changes or revisions were justified. Only the arid desert regions were affected.

We are pleased to announce the new rainfall numbers led to updating our manual with an addendum. Briefly, this addendum addresses the Antecedent Moisture Condition (AMC) for arid regions of the County. The attached map, ADD-1, identifies where AMC I may now be used.

Please use the attached link to our website to access the addendum (March 2010); also included is a list of Frequently Asked Questions (FAQ's) and a map of the affected area. A GIS version of the map is also available.

If you have any questions, please contact Mike Fox, Chief-Water Resources Division at (909) 387-8213.

Sincerely,

KEVIN B. BLAKESLEE, P.E.
Deputy Director-Flood Control

KBB:MJF:bfb

County of San Bernardino Hydrology Manual Addendum for Arid Regions

April 2010

I. INTRODUCTION

After publication of the NOAA Atlas 14 rainfall atlas and the associated data base (NOAA, 2004, revised 2006), the County of San Bernardino Water Resources Division assessed the new publication towards the possibility of updating its Hydrology Manual (1983, revised 1986), particularly in the arid regions of the County. NOAA Atlas 2 (NOAA, 1973) served as the basis for the San Bernardino Hydrology Manual dated 1986. The updated NOAA Atlas 14 publication includes data from several rain gages which were not available at the time of the prior publication of NOAA Atlas 2, as well as 25 years of additional data at several of the rain gages used in NOAA Atlas 2. Consequently, thousands of additional station years of data are included in the updated NOAA Atlas 14. Upon assessing the new NOAA Atlas 14 rainfall statistics and mapping, the County updated their Hydrology Manual criteria to reflect the changes in rainfall statistics and trends developed with NOAA Atlas 14. This Addendum provides a summary of these updated criteria.

It is noted that numerous rain gages found in the NOAA Atlas 14 study area are not included in the NOAA Atlas 14 update and therefore care is needed when applying the updated Hydrology Manual criteria. Hydrology studies need to consider all available rainfall data by identifying rain gages located near or in the vicinity of the study area and need to obtain and review the relevant rainfall data. Such additional rainfall information includes, but is by no means limited to: NOAA (<http://www.nws.noaa.gov/>), CA-DWR (<http://cdec.water.ca.gov/>), CIMIS (<http://www.cimis.water.ca.gov/cimis/welcome.jsp>), as well as gage data available from San Bernardino County. The results of such a review should be compared with the NOAA Atlas 14 results and a determination made as to the appropriateness in using the NOAA Atlas 14 results or whether a re-assessment of all rainfall data relevant to the study area should be made. Such determinations and reviews must be coordinated with the County in order to conclude the most appropriate rainfall statistics to use, including assessments of station record length and quality, among other factors.

The primary topics considered in the Addendum are:

1. Rainfall quantities for various peak durations of rainfall, and related return periods;
2. Antecedent Moisture Conditions (or "AMC") used in hydrology studies for design and planning;
3. Soil Grouping designations and related maps.

II. RAINFALL STATISTICS

The County of San Bernardino Hydrology Manual (1986) contains isohyetal curves developed for estimating the 2-year return frequency values for the peak 6- and 24-hour durations of rainfall, the 10-year 1-hour rainfalls, and the 100-year 1-hour, 6-hour and 24-hour rainfalls. These isohyetal maps are based upon use of the NOAA Atlas 2 (1973) information. The NOAA Atlas 14 provides information for various peak durations of rainfall depths and for various return periods (return frequencies), including all of the key durations and return periods detailed in the Hydrology Manual.

Access to the NOAA Atlas 14 information is found at <http://hdsc.nws.noaa.gov/hdsc/pfds/>.

Another resource available for assessing rainfall for hydrology studies is the depth duration frequency studies developed by the California Department of Water Resources (DWR). Some of the gages analyzed by DWR are not included in the NOAA Atlas 14 and should be considered for appropriateness in studies submitted to the county. The depth-duration frequency tables can be obtained as Microsoft Excel files from the DWR website at the following address:

http://www.water.ca.gov/floodmgmt/hafoo/csc/climate_data/.

It is noted that the Hydrology Manual provides interpolation methods for development of rainfall estimates for the 5-minute, 30-minute, 1-, 3-, 6-, and 24-hours of peak rainfall, including recommendations regarding log-log slopes of the relevant mass rainfall plots (for example, see Hydrology Manual Figures E-36 through E-45). The NOAA Atlas 14 provides estimates for these peak durations of rainfall depths directly in tabular form, on a rain gage by rain gage basis (for those gages used in the NOAA Atlas 14 analysis). Hydrology studies prepared using this Addendum should develop the relevant rainfall quantities required for the Hydrology Manual using the newer NOAA Atlas 14 estimates and, if available, the DWR estimates to assess the appropriate rainfall quantities to be used. Additionally, the study should consider all other rain gage information available in the proximity of the study watershed. The submittal should consider these several forms of rainfall information and provide a recommendation as to the appropriate rainfall information to use.

III. ANTECEDENT MOISTURE CONDITIONS (AMC)

The Antecedent Moisture Condition (AMC) concept is a classification of the watershed runoff conditions and is related to the prior five-day precipitation. By examining this prior five-day rainfall, the watershed can be categorized as being wet, average or dry. This classification of the watershed impacts the runoff which can be expected during a particular storm event. Original literature regarding AMC conditions were published by the Soil Conservation Service (SCS) in 1964 in the National Engineering Handbook,

Section 4. (The SCS had since changed to be the Natural Resources Conservation Service (NRSC).) In the 1993 update to the National Engineering handbook, the NRSC revised the AMC concept to that of Antecedent Runoff Condition (ARC), where ARC values correspond to statistical envelopments of the relevant rainfall-runoff information, versus the AMC concept correlating to contemplated prior moisture conditions of the watershed. Similar to many other agencies, the County continues to use the AMC approach in order to determine runoff quantities appropriate for design and planning purposes. The AMC approach should be used in all hydrologic studies prepared for County review or approval as presented in the Hydrology Manual (1986), without modification.

Based on the NOAA Atlas 14 statistical data, updated AMC designations for use in arid region hydrology studies are as shown in Addendum Figures ADD-1. It is noted that the NOAA Atlas 14 did not include all available rain gages, and therefore the hydrology study should examine other relevant rainfall gages to assess the appropriateness of the AMC designations shown in Addendum Figures ADD-1. Regional or Master Plan studies should consider all sources of information. The AMC condition used for these studies must be approved by the County.

IV. SOIL GROUPING DESIGNATIONS

The soil grouping information contained in Section C of the Hydrology Manual (1986) has been updated and can be accessed at <http://websoilsurvey.nrcs.usda.gov/app/WebSoilSurvey.aspx>. Use of this information follows the directions provided in the Hydrology Manual (1986).

V. REFERENCES

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